1	CHAPTER 25
2 3 4	SEPTIC TANK <u>S</u> , <u>AND/OR</u> -SOIL ABSORPTION SYSTEMS, AND OTHER SMALL WASTEWATER SYSTEMS
5 6 7	Section 1. General Authority.
8 9	This rule is promulgated pursuant to Wyoming Statutes (W.S.) 35-11-101 through 35-11-1904, specifically 35-11-302(a)(iii).
10 11	Section 2. Definitions Objective.
12 13 14 15 16 17 18	(formerly Section 1) This part Chapter contains the minimum standards for the design and construction of sewerage small wastewater systems, treatment works and disposal systems for domestic wastes and industrial wastes generated by facilities other than specifically covered by other parts of this Chapter which are defined by W.S. 35-11-103(c)(ix). In addition, this Chapter contains the minimum standards for the design and construction of Underground Injection Control (UIC) Class V facilities 5C1-5C3, 5C6, 5D1, 5E1, 5E3-5E5 as defined in Chapter 16, Appendices A and B.
20 21 22 23 24 25 26	The following situations will require the application package to be sealed, signed, and dated by a professional engineer (PE): non-domestic wastewater from commercial and industrial facilities, high strength wastewater, individual permits to construct, or standard soil absorption systems with a soil percolation rate which is either less than 5 minutes per inch (mpi) or more than 60 minutes per inch (mpi).
26 27 28 29 30	These standards pertain to permits required pursuant to Chapters 3 and 25, Wyoming Water Quality Rules and Regulations. The installation of all components of a small wastewater system require a permit to construct. Permits to construct are specified throughout this chapter as general permits, described in Chapter 3, Section 7; permit by rule, described in Chapter 3, Section 8; or as
31	individual permits to construct, described in Chapter 3, Section 6.
32 33	Section 3. Design Flows Definitions.
34 35 36 37 38	(a) "Absorption system" means a system constructed under the surface of the ground which receives and distributes effluent from a pretreatment device effectively filtering the effluent through soil or media. "Absorption surface" means the interface where treated effluent infiltrates into native or fill soil.
39 40 41 42	(b) "Aerobic unit" means a covered, watertight receptacle which receives wastewater. The unit removes settleable solids, floatable material, and a part of soluble organic matter by the use of aerobic biological treatment. "Bed" means a soil treatment and dispersal system where the
43 44 45 46	width is greater than three (3) feet.  (c) "Building drain" means the building drain is that part of the lowest piping of a drainage system which receives the discharge from soil, waste and other drainage pipes inside the walls of the building and conveys it to the building sewer beginning two feet (.6m) outside the
47 48 49	building wall. "Bedrock" means geological layers, of which greater than 50 percent by volume consist of unweathered in-place consolidated rock or rock fragments. Bedrock also means weathered in-place rock which cannot be hand augered or penetrated with a knife blade.

(d) "Bedroom" means any room that is or may be used for sleeping.

- (e) "Blackwater" means water containing fecal matter and/or urine.
- (f) "Dosing system" means the system of tanks, pumps or syphons, and piping located between the septic tank and soil absorption system which is intended to apply a large quantity of settled wastewater to the absorption system in a short period of time. "Five day biochemical oxygen demand (BOD<sub>5</sub>)" means a measurement of the dissolved oxygen used by microorganisms in the biochemical oxidation of organic matter during a five (5) day period.
- (g) "Hydrogeological study" means a study of the occurrence, distribution, quality and movement of the shallowmost groundwater of the site and the potential impact of wastewaters on the groundwater.

Previously 2(d) "Building sewer" means the building sewer is that part of the horizontal piping the pipe of a drainage system which extends from the end of the building drain and conveys the building drain discharge to the septic tank or other onsite sewage disposal facility carries wastewater from the building.

- (h) "Impermeable soil" means any soil which has a percolation rate greater than 60 minutes per inch. "Chamber" means a domed open bottom structure that is used in lieu of perforated distribution pipe and gravel media.
- (i) "Pump Tank" means a tank in which the dosing pumps or syphons are installed. "Delegated small wastewater program" means a local governmental entity, delegated by the Administrator, with the authority to administer the provisions of W.S. 35-11-301(a) (iii) for small wastewater systems pursuant to the provisions of W.S. 35-11-304.
- (j) "Direct human consumption food crops" are crops consumed directly by humans. These include, but are not limited to fruits, vegetables and grains grown for human consumption.
- (k) "Domestic wastewater" means a combination of the liquid or water-carried wastes from residences, business buildings, institutions, and other establishments arising from normal living activities.

Previously 2(e) (1) "Domestic sewage septage" "Domestic" means the liquid—and waterborne wastes or solid material derived removed from the ordinary living processes a waste treatment vessel, free from industrial wastes, and of such character as to permit satisfactory disposal without special treatment that has received only wastes from residences, business buildings, institutions, and other establishments arising from normal living activities.

- (m) "Dosing tank" means a tank equipped with an automatic siphon or pump designed to discharge effluent on an intermittent basis.
- (n) "Effluent" means a liquid flowing out of a septic tank, other treatment vessel or system.

(o) "Effluent filter" means a removable, cleanable device inserted into the outlet piping of a septic tank or other treatment vessel designed to trap solids that would otherwise be transported to the soil absorption system or other downstream treatment components.

- (p) "Evapotranspiration" means the combined loss of water from soil by evaporation from the soil or water surface and by transpiration from plants.
- (q) "Greywater" means untreated wastewater that has not been contaminated by any toilet discharge, which is unaffected by infectious, contaminated, or unhealthy bodily wastes, and does not present a threat from contamination by unhealthful processing, manufacturing, or operating wastes. "Greywater" includes but is not limited to wastewater from bathtubs, showers, washbasins, clothes washing machines (unless soiled diapers are serviced), laundry tubs, and kitchen sinks.
- (r) "Grease interceptor" means a device designed to separate fats, oils, and grease from wastewater.
- (s) "Groundwater" means subsurface water that fills available openings in rock or soil materials such that they may be considered water saturated under hydrostatic pressure.
- (t) "High groundwater" means seasonally or periodically elevated levels of groundwater.
- (u) "High strength wastewater" means a wastewater stream with a  $BOD_5$  higher than 200 mg/L.
- (v) "Holding Tank" means a watertight receptacle designed to receive and store wastewater.
- (w) "Manifold" means a non-perforated pipe that distributes effluent to individual distribution pipes.
- (x) "Mound system" means an onsite wastewater system where the bottom of the absorption surface is above the elevation of the existing site grade, and the absorption surface is contained in a mounded fill body above the grade.
- (y) "Mulch basin" means an excavated area that has been refilled with a highly permeable media, organic and inorganic materials intended to distribute greywater to irrigate vegetation.
- (z) "Pathogens" are disease-causing organisms. These include, but are not limited to, certain bacteria, protozoa, viruses, and viable helminth ova.
- (aa) "Percolation rate" means the time expressed in minutes per inch required for water to seep into saturated soil at a constant rate.
- 143 (bb) "Pipe invert" means the bottom or lowest horizontal point of the internal surface of the pipe.

  144 the pipe.

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146 (cc) "Percolation test" means the method used to measure the percolation rate of water 147 into soil as described in Appendix A. 148 149 (dd) "Permit by rule" means an authorization included in these rules which does not 150 require either an individual permit or a general permit. A facility which is permitted by rule must 151 meet the requirements found in this chapter, but is not required to apply for and obtain a permit to 152 construct and operate the facility. 153 154 (ee) "Pressure distribution" means a network of pipes in which effluent is forced 155 through orifices under pressure. 156 157 "Restrictive layer" means a nearly continuous layer that has one or more physical, 158 chemical, or thermal properties that significantly impede the movement of water and air through 159 the soil or that restrict roots or otherwise provide unfavorable root conditions. Examples are 160 bedrock, cemented layers, dense layers, and frozen layers. 161 162 (gg) "Septage" means liquid or solid material removed from a waste treatment vessel 163 that has received wastes from residences, business buildings, institutions, and other 164 establishments. 165 166 (hh) "Septic tank" means a buried, watertight tank designed and constructed to receive and treat raw wastewater. 167 168 169 "Service provider" means a person authorized and trained by a system 170 manufacturer or their vendor to operate and maintain any proprietary system. 171 172 "Soil absorption system" means a shallow, covered, excavation made in 173 unsaturated soil into which wastewater effluent from the septic tank is discharged through 174 distribution piping for application onto absorption surfaces through porous media or 175 manufactured components placed in the excavations. 176 177 "Trench" means an absorption surface with a width of three (3) feet or less. (kk) 178 179 **Section 4. Isolation Design Flows.** 180 181 (formerly Section 3) The sewerage system, treatment works and disposal system shall have a 182 minimum absorption area based on the minimum peak design flows listed in Table 1 below. The 183 volume of wastewater shall be determined by one of the following: 184 185 (a) Tables 1 and 2 provided in this section. 186 187 (b) Metered water supply data from the facility. 188 189 (c) Metered water supply data from another facility where similar water demands have 190 been demonstrated. 191

## Table 1

### **Quantities of Domestic Sewage Flows**

Type of Establishment	Flow (gallons per day per)			
Residential Units				
Single Family Dwellings	<del>150/</del> bedroom			
Multiple Family Dwelling (with laundry capabilities)	150/bedroom			
Multiple Family Dwelling (without laundry capabilities)	120/bedroom			
Cottages	<del>50/person</del>			
Mobile Home Parks	350/home*			
Commercial Facilities				
Airports	4/ <del>passengers</del>			
Bar	<del>3/patron</del>			
Bathhouses and swimming pools	10/person			
Campgrounds (individual sewer outlets available)	100/site			
Campgrounds (service building only)	<del>75/site</del>			
Car or truck wash	200/vehicle			
Church (no food preparation and/or dishwashing)	<del>5/seat</del>			
Church (food preparation and/or dishwashing)	<del>7/seat</del>			
Country Club	<del>100/</del> member			
Factories	30/employee			
Hospital	200/bed			
Laundry (self-service)	600/machine or 50/cycle			
Motels	80/double bed or 40/single bed			
Office building	<del>30/employee</del>			
Restaurant (toilet and kitchen wastes)	<del>13/meal</del>			
Restaurant (kitchen wastes)	<del>6/meal</del>			
Restaurant (additional for bars and lounges)	<del>2/meal</del>			
Restaurant (kitchen wastes with disposable service)	<del>2/meal</del>			
Rest Home	100/resident			
Schools Boarding	100/resident student			
Day, without gyms, cafeterias, or showers	15/student			
Day, with cafeterias only	<del>20/student</del>			
Day, with cafeteria, gym and showers	25/student			
Service stations	10/vehicle served			
Shopping Center	2/parking space			
Store, Retail	<del>30/employee</del>			
Theaters: Movie	<del>5/</del> seat			
<del>Drive In</del>	15/vehicle space			
Warehouses	<del>30/employee</del>			

<sup>\*</sup> Must consider flow into the soil absorption system from mobile homes where taps are allowed to run to prevent freezing.

Table 1. Residential Design Flow Rates per Bedroom (gallons per day, gpd)<sup>1</sup>

1 bedroom	<u>150</u>
2 bedrooms	<u>280</u>
3 bedrooms	<u>390</u>
4 bedrooms	<u>470</u>
<u>5 bedrooms</u>	<u>550</u>
<u>6 bedrooms</u>	<u>630</u>

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Table 2. Non-Residential Wastewater Design Flow Rates<sup>1</sup>

Facility	Unit	Flow (gallons/unit/day)
Airports	<u>person</u>	4
<u>Apartment</u>	<u>bedroom</u>	<u>120</u>
Automobile Service Station	vehicle served	10
Bar <u>s</u>	<u>seat</u>	<u>20</u>
Bathhouses and swimming pools	person	10
Campgrounds (w/ toilets only)	<u>person</u>	<u>25</u>
Campgrounds (w/shower facility)	<u>person</u>	<u>45</u>
<u>Church</u>	<u>person</u>	4
Country Club	member	<u>25</u>
Day School, Office Building, Retail Store, Warehouse (no showers)	person	<u>15</u>
Hospital	bed	<u>250</u>
Industrial Building (sanitary waste only)	employee	<u>20</u>
Laundry (self-service)	<u>machine</u>	<u>450</u>
Mobile Home	<u>bedroom</u>	See table 1
Motel, <u>Hotel, Resort</u>	<u>bedroom</u>	<u>140</u>
Recreational Vehicle	<u>each</u>	<u>100</u>
Rest Home, Care Facility, Boarding School	bed	100

<sup>&</sup>lt;sup>1</sup>An unfinished basement is considered two (2) additional bedrooms.

<sup>2</sup>The design flow shall be increased by eighty (80) gpd for each additional bedroom over six (6).

Restaurant	meal	<u>10</u>
Restaurant (kitchen waste only)	<u>meal</u>	<u>6</u>
Theater	<u>seat</u>	<u>3</u>

<sup>1</sup>Values shown in the above table are the typical flow rates from *Wastewater Engineering Treatment and Reuse*, Metcalf and Eddy, 2003 Edition.

### Section 5. Site Suitability Systems not Specifically Covered by This Rule.

This section is provided to encourage new technology and equipment and provide a process for evaluating and permitting designs which deviate from this rule. The proposed construction of facilities and processes not in compliance with this rule may be permitted provided that the facility, when constructed and operated, meets the objective of these rules.

- (a) Each application for a permit to construct shall include an engineering design report, detailed construction plans, and technical specifications for all piping, tanks, and equipment. All of the documents shall have a suitable title showing the owner's name and the Wyoming registration number, seal, and signature of the engineer.
- (b) Each application for a permit to construct will be evaluated on a case-by-case basis using the best available technology. The application shall include at least one of the following:
- (i) Data obtained from a full scale, comparable installation that demonstrates the acceptability of the design.
- (ii) Data obtained from a pilot plant operated under the design condition for a sufficient length of time to demonstrate the acceptability of the design.
- (iii) Data obtained from the theoretical evaluation of the design that demonstrates a reasonable probability the facility will meet the design objectives.
- (iv) An evaluation of the flexibility of making corrective changes to the constructed facility in the event it does not function as planned.
- (c) If an applicant wishes to construct a pilot plant to provide data necessary to show the design will meet the purpose of the act, a permit to construct must be obtained.

### Section 6. Building Sewer Pipes Site Suitability.

(a) (Formerly 4(c) Location. Small wastewater systems must be located where the surface drainage is sufficient to allow proper operation of the small wastewater system. Formerly 10 (a)(iii) Runoff. Surface runoff shall be diverted around or away from all soil absorption systems. Avoid depressions and bases of slopes and areas in the path of runoff from roofs, patios, driveways, or other paved areas unless surface drainage is provided. Formerly 4(c))Absorption systems Small wastewater systems shall not be located beneath buildings, parking lots, roadways, driveways, irrigated landscaping, or other similarly compacted areas.

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(formerly 10(a)(i))(b) Replacement area. An area shall be designated and shown on the plans for future installation of a replacement absorption system. The site must include area for both the proposed soil absorption system and a future replacement soil absorption system. Both the proposed and replacement soil absorption systems shall be sized to receive one-hundred (100%) percent of the wastewater flow. If a trench system is used, the replacement area soil absorption system may include the area be located between the trenches-of the proposed soil absorption system if sufficient spacing has been provided there is at least nine (9) feet of spacing between trench sidewalls. At least three feet of undisturbed soil shall remain between the existing and replacement trench side walls.

(formerly 5(c)) Groundwater protection and bedrock or impermeable soil separation.

(formerly 5(c)(i)) (c) For single family homes, For standard soil absorption systems the effective suitable soil depth to bedrock or impermeable soil must be shall extend at least four (4) feet-from below the bottom of the absorption system-stone and the natural ground surface to any restrictive layer, fractured rock, or highly permeable material.

(formerly 5(e)(i))(d)—The depth to-seasonally high groundwater-must shall-be at least four (4) feet-from below-the bottom of the absorption-system-surface-stone and at least two feet from the natural ground surface. for all treatment systems except pressure distribution. (formerly 5(e)(ii))—For all systems other than single family homes up to 2000 gallons per day, the depth to bedrock or impermeable soil must be at least four feet from the natural ground surface. The depth to seasonally high groundwater must be at least four feet from the bottom of the absorption system stone and at least two feet from the natural ground surface. Also, For pressure distribution systems, the depth to high groundwater shall be a minimum of at least three (3)—feet of unsaturated soil shall be maintained between-below the-bottom of the absorption-system stone surface and the estimated groundwater mound imposed on the seasonally high groundwater table. if the percolation rate of the soil is five (5) minutes per inch or greater (5-60 mpi). The height of the groundwater mound may be estimated from Figures 1 through 6. The average daily flow should be used and may be estimated as 0.6 times the flow determined from Table.

(formerly 5(c) (iii)) For all systems larger than 2000 gallons per day, a minimum of three feet of unsaturated soil shall be maintained between the bottom of the absorption system stone and the estimated groundwater mound imposed on the seasonally high groundwater table. The maximum height of the groundwater mound shall be estimated by the design engineer.

(formerly Section 5(e))(e) Sloping ground installations Slope

(formerly Section 5(e)(i))(i) Absorption systems shall not be located in an area where the natural slope is steeper than stated below. The following are Table 3 shows the maximum permissible slopes on which an absorption system may be constructed.

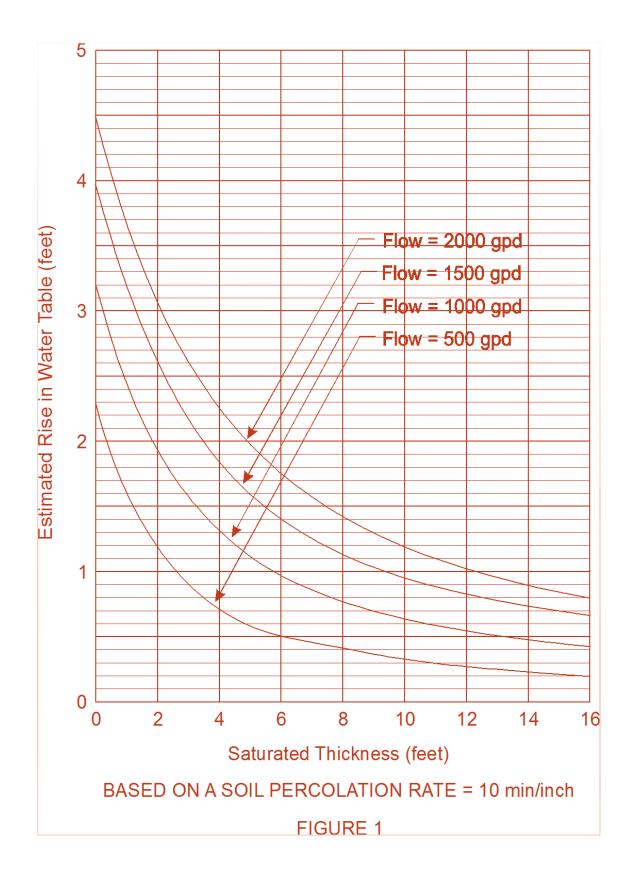
# (formerly contained in Section 5(e)(i) Table 3. Slope and Percolation Rates for Absorption Systems

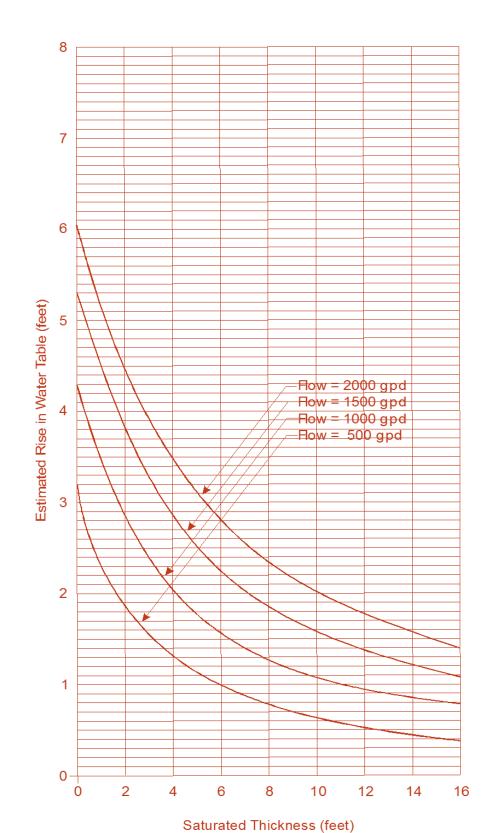
Percolation Rate (minutes/inch)	Maximum Slope <sup>1</sup>
Faster than 5	25%
6-45	20%
46-60	15%

<sup>\*1</sup> Flatter slopes may be required where the effluent may surface sufaces downslope.

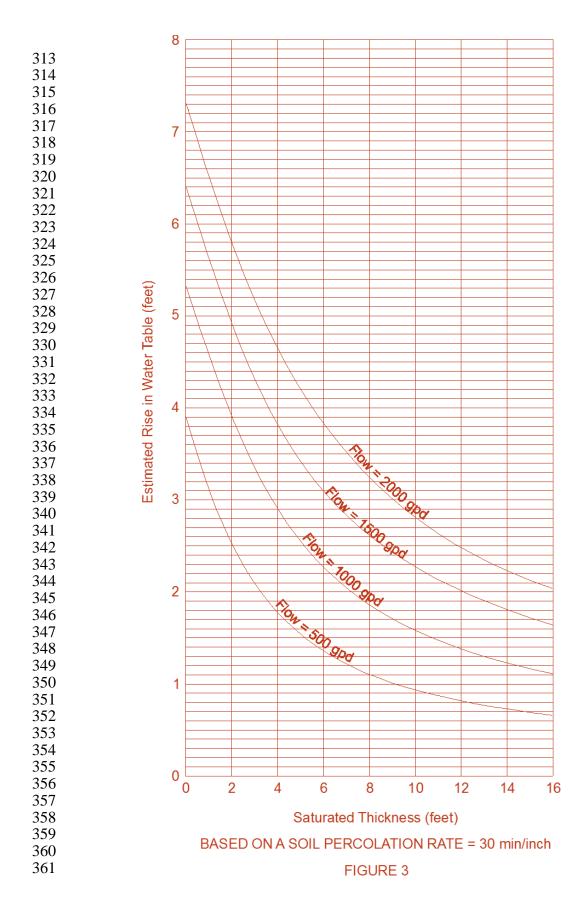
(formerly 5(e)(ii) "Saturated thickness" Distance between the seasonally high groundwater table and the under lying impervious layer such as clay, bedrock or soils with significantly lower permeability.

(formerly 5 (e)(ii)) "Estimated Rise in Water Table": The estimated distance the water table will rise at the center of the absorption system above the initial water table when the indicated flow is applied daily.



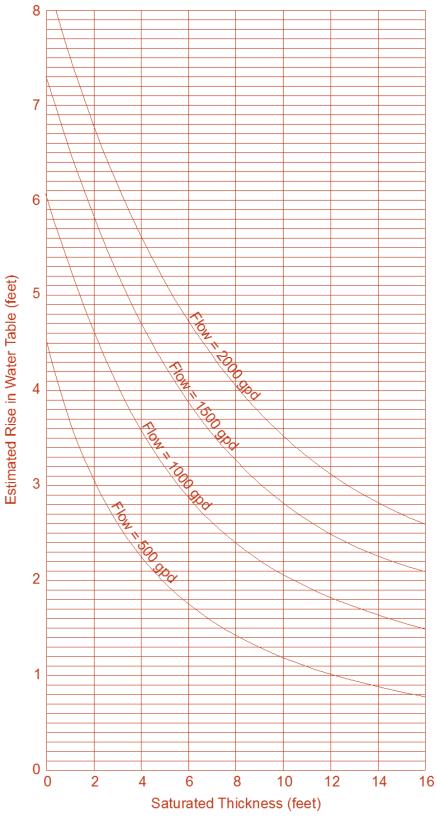


BASED ON A SOIL PERCOLATION RATE = 20 min/inch
FIGURE 2



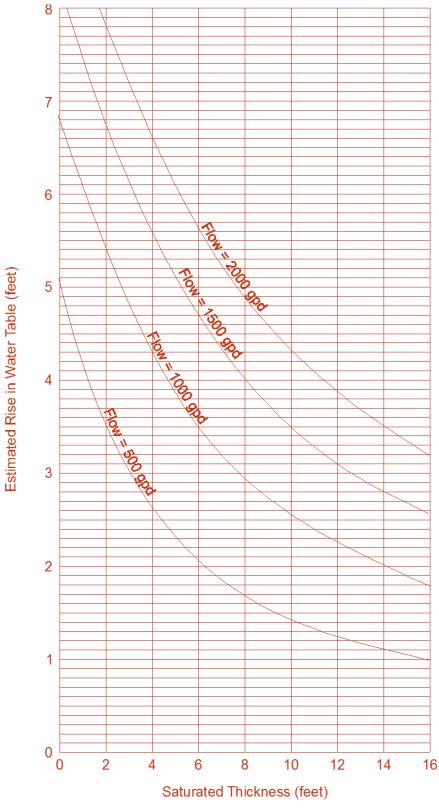
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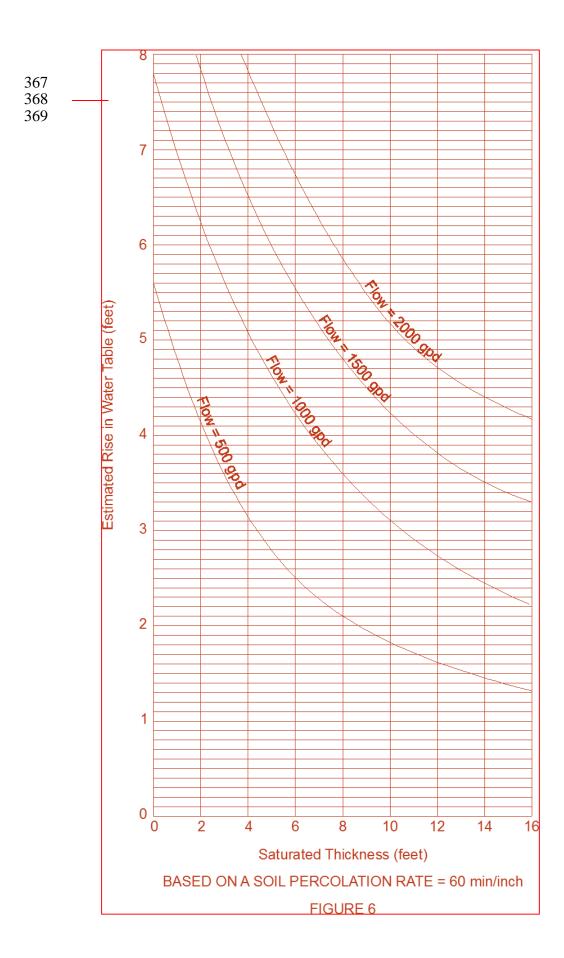


BASED ON A SOIL PERCOLATION RATE = 40 min/inch FIGURE 4





BASED ON A SOIL PERCOLATION RATE = 50 min/inch FIGURE 5



371 preferred installation method for sloping terrain. The bottom of individual trenches shall be level 372 and the trenches shall be constructed to follow the contours of the land. 373 374 (iii) The placement of multiple trenches, with each subsequent trench down slope of 375 the previous trench shall be avoided when the addition of effluent to the soil absorption system 376 trenches may lead to either an unstable slope or seepage down slope. 377 378 (formerly 5(e)(ii))(iv) All absorption surfaces must be located at least 15 379 horizontal feet from the top of any break in slope which exceeds the maximum allowed in 380 subsection (i) above slope allowed. 381 382 (f) Soil Exploration Pit and Percolation Tests 383 384 Delegated small wastewater programs shall require a percolation test in 385 addition to the soil exploration pit. 386 387 (formerly 5(a)) Soil exploration. Soil exploration A minimum of one soil 388 exploration pit within the proposed soil absorption system location shall be excavated, to a 389 minimum depth of four (4) feet below the bottom of the proposed soil absorption system shall be 390 made to provide information on subsoil conditions to evaluate the subsurface conditions. 391 392 (formerly 5(b)) Soil evaluation. 393 394 (formerly 5(b)(i)) No less than three percolation tests shall be run in the 395 proposed absorption system location. The percolation tests shall be performed in accordance with Appendix A of this part. The type of soil encountered at the percolation test location shall be 396 397 specified. 398 399 ((formerly 5b)(ii)) (iii) The percolation test shall be performed in accordance with 400 Appendix A of this chapter. An evaluation of the soil texture, in the proposed soil absorption 401 system location, by a person experienced in soils classification, may be used as an additional tool 402 to confirm the percolation rate. but at least one percolation test shall be performed. 403 404 (formerly Section 4)(g) Isolation Minimum horizontal setback distances (in feet) are as 405 follows: 406 407 (formerly 4(a)) Domestic wastewater. The isolation distances listed below apply when 408 domestic wastewater is the only wastewater present. 409 410 (formerly 4(a)(i)) If the flow is less than 2000 gallons per day (gpd), the 411 minimum isolation distance (in feet) shown in Table 2 shall be maintained. 412 413 414 415 416 417

Serial distribution, with the use of drop boxes or approved fittings, is the

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# (formerly Table 2) <u>Table 4. Minimum Horizontal Setbacks for Domestic</u> Wastewater<sup>1, 2</sup>

From	To Septic Tank Or Equivalent	To Absorption System
Wells (includes neighboring wells)	50	100
Public Water Supply Well	<u>100²</u>	<u>200²</u>
Property Lines	10	10
Foundation Wall (w/o drains)	5	10
Foundation Wall (with drains)	5	25
Potable Water Pipes	25	25
Septic Tank	<u>N/A</u>	10
Stream or Surface Body of Water, Spring (including seasonal and intermittent)	50	50
<u>Cisterns</u>	<u>25</u>	<u>25</u>

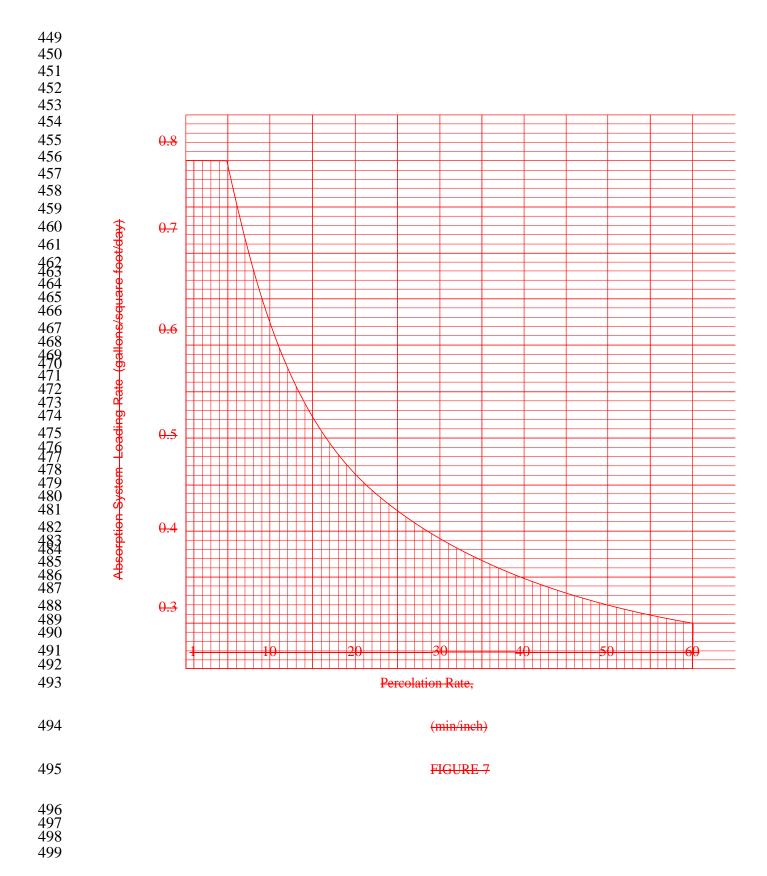
<sup>1</sup> (formerly 4(b)) Non-domestic wastewater. For disposal of wastewaters other than domestic non-domestic wastewater, the isolation setback distance shall be determined by a hydrogeological study in accordance with Section 15 17(b) of Chapter 3, but shall not be less than the distances shown in Table 4.

<sup>2</sup> Small wastewater systems that discharge to the same aquifer that supplies a public water supply well and are located within Zone 1 or 2 (Attenuation) of the public water supply well, as determined by *Wyoming Department of Environmental Quality Source Water Assessment Project* (2004) or as established in Section 2 of the *Wyoming Wellhead Protection Guidance Document* (1997), shall provide additional treatment. These systems will be required to obtain an individual permit to construct and will require that a PE sign, stamp, and date the application, as stated in Section 2 of this chapter. The additional treatment shall be in accordance with Chapter 3 Section 2(b)(ii). The treatment shall reduce the nitrates to less than 10 mg/L of NO<sub>3</sub>- as N and provide 4-log removal of pathogens before the discharge leaves the property boundary of each small wastewater system.

### Section 7. Soil Absorption System Sizing.

(a) Trench, bed and seepage pit systems. The total infiltrative infiltration surface of a soil absorption system shall be calculated based on the flow rate as determined by the criteria stated in Section 3 and with the allowable loading rate as determined by using Figure 7. by dividing the design flow rates (gpd) from Table 1 or Table 2 by the loading rate (gpd/ft²) found in Table 5. The total infiltrative surface is the sum of the sidewall and bottom areas of the absorption system below the invert of the distribution pipe.

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<u>Table 5. Rates of Wastewater Application for Soil Absorption System Areas</u>

<b>Percolation Rate</b>	<b>Loading Rate</b>	<b>Percolation Rate</b>	<b>Loading Rate</b>
<u>(mpi)</u>	$(\mathbf{gpd}/\mathbf{ft}^2)$	<u>(mpi)</u>	$(gpd/ft^2)$
<u>5</u>	<u>0.80</u>	<u>21</u>	<u>0.45</u>
<u>6</u>	<u>0.75</u>	<u>22</u>	<u>0.44</u>
<u>7</u>	<u>0.71</u>	<u>23-24</u>	<u>0.43</u>
<u>8</u>	<u>0.68</u>	<u>25</u>	<u>0.42</u>
<u>9</u>	<u>0.65</u>	<u>26-27</u>	<u>0.41</u>
<u>10</u>	<u>0.62</u>	<u>28-29</u>	<u>0.40</u>
<u>11</u>	<u>0.60</u>	<u>30-31</u>	<u>0.39</u>
<u>12</u>	<u>0.58</u>	<u>32-33</u>	<u>0.38</u>
<u>13</u>	<u>0.56</u>	<u>34-35</u>	<u>0.37</u>
<u>14</u>	<u>0.54</u>	<u>36-37</u>	<u>0.36</u>
<u>15</u>	<u>0.52</u>	<u>38-40</u>	<u>0.35</u>
<u>16</u>	<u>0.50</u>	<u>41-43</u>	<u>0.34</u>
<u>17</u>	<u>0.49</u>	<u>44-46</u>	<u>0.33</u>
<u>18</u>	<u>0.48</u>	<u>47-50</u>	<u>0.32</u>
<u>19</u>	<u>0.47</u>	<u>51-55</u>	<u>0.31</u>
<u>20</u>	<u>0.46</u>	<u>56-60</u>	<u>0.30</u>

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(b) Soils with a percolation rate of 60 minutes per inch or greater are unacceptable for standard absorption systems. The total infiltration area shall be defined as follows:

507 (i) For standard trenches the total infiltration area shall be calculated based on the
508 following formula:
509
510

A= L(W + 2S)

A = L(W + 2S)

A = Total infiltration area

L = Total length of trench

515 516 <u>W = Bottom width</u> 517

518 S = Sidewall height of 12 inches or less 519

(A) The sidewall height is the depth below the flowline of the pipe to the bottom of the trench.

(B) The maximum credit for sidewall height shall not exceed twelve (12)

inches even if the actual sidewall height exceeds twelve inches.

(ii) For chamber trenches, the total infiltration area shall be calculated based on the following formula:

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531	$\underline{\qquad \qquad A = Total\ infiltration\ area}$
532	
533	L = Total length of trench
534 535	E = Effective bottom width (Multiply width of the chamber by factor of 1.43 to
536	get effective bottom width)
537	get effective bottom widthy
538	S = Sidewall height of 12 inches or less
539	
540	(A) The factor of 1.43 incorporates a thirty percent (30%) reduction of the
541	bottom area.
542	
543	(B) The maximum credit for sidewall height shall not exceed twelve (12)
544	inches even if the actual sidewall height exceeds twelve (12) inches.
545	
546	(C) The sidewall height is the height of the slotted sidewall of the chamber or
547	depth below the flow line of the inlet pipe, whichever is less.
548	
549	(iii) For bed systems, the total infiltration area shall be calculated based on the
550	following formula:
551 552	$\underline{\hspace{1cm} A = LW}$
553	A = Total infiltration area
554	A – Total illituation area
555	L = Total length of bed
556	L = Total length of sea
557	W = Width of the bed.
558	
559	(A) The sidewall credit shall not be used in calculating the total infiltration area
560	for a bed system.
561	
562	(formerly 5(d))(c) Excessively permeable soils. Coarse sand or soils having a
563	percolation rate of less than one (1) minute per inch (mpi) or less are unsuitable for subsurface
564	<u>effluent sewage</u> disposalThese soils may be used if a <u>six inch a one (1) foot layer of soil fine</u>
565	sand or loamy sand having a percolation rate of five minutes per inch or greater is placed between
566	the leach system stone and the existing soil below the constructed soil absorption system. The
567	soil absorption system shall be sized-based on the percolation rate of the fill material.
568	
569	Section 8. (formerly Pretreatment) Building Sewer Pipes.
570	
571	(formerly 6(a)) Building drain pipe. All building drain pipe shall comply with the
572	standards published in the Uniform Plumbing Code 1982 or other locally approved, nationally
573	recognized plumbing code.
574	
575	(formerly 6(b)) Building sewer pipe. All building sewers shall be installed in accordance
576	with the Uniform Plumbing Code-1982 or other locally approved, nationally recognized
577	plumbing code 2012 International Plumbing Code (IPC). In the absence of an a locally approved
578	plumbing code, and in addition to the IPC, the building sewer shall comply with the following:
579	

(formerly 6(b)(i)) (a) (Material) Suitable building sewer pipe materials are Polyvinyl Chloride (PVC) or Acyrlonitrile—Butadiene-Styrene (ABS) cast or ductile iron, portland cement, or vitrified clay pipe shall be used for sewer pipes. The septic tank inlet and outlet pipes shall be cast or ductile iron or schedule 40 PVC or ABS pipe and shall-extend past the septic tank excavation to solid ground span the excavations for the septic tank and/or dosing chamber.

American Society for Testing and Materials (ASTM) D-3034 Standard Dimension Ratio (SDR) 35 plastic pipe may be used if the void at the tank's side is filled with material which is granular, clean and compacted.

(formerly 6(b)(ii)) (b) Size. Building sewer pipes shall not be smaller than four inches in diameter. They shall be sized to handle the peak hourly flow from the building.

Building sewer pipes shall be sized to handle the peak hourly flow from the building and shall not be smaller than four (4) inches in diameter. When two different sizes or types of sewer pipes are to be connected, a proper type of fitting or conversion adapter shall be used.

(c) Sewer pipe shall not decrease in size flowing downstream.

(formerly 6(b)(iii))(d) Slope. Building sewer pipes should shall be laid at a minimum standard slope of 1/4 inch per foot, but and shall not be flatter than 1/8 inch per foot.

(formerly 6(b)(iv)) Alignment. Building sewer pipes should be laid in a straight line. Any single change or cumulative change of alignment of 22 ½ degrees or greater shall be served by a cleanout.

(formerly 6(b)(v))(e) Cleanouts. Cleanouts shall be provided at branch connections, every change in alignment, and at least every 100 feet in straight runs maximum.

(formerly 6(b)(vi))(f) Backfilling. All sewer piping shall be laid on a firm bed throughout its entire length. It shall be protected from damage due to rocks, hard lumps of soil, debris and the like.

(formerly 6(b)(vi))(g) Special care shall be utilized used to prevent lateral movement or ovalation deformation during backfill. The backfill material shall be compacted to a density at least equivalent to the trench walls. Backfill over the pipe shall be of sufficient depth to protect the pipe from expected traffic loads and the wastewater from freezing.

Section 9. Dosing Systems Following Septic Tanks Septic Tanks and Other Treatment Tanks.

(formerly 8(a)) (a) Septic tanks.

(formerly 8(a)(i)) (i) Material. The Septic tanks shall be fabricated or constructed of durable concrete, fiberglass or an approved material not subject to excessive corrosion or decay and structurally capable of supporting the loads to which it will be subjected. The tank shall be water tight. The Tanks shall be water tight and fabricated to constitute an individual structure, and shall be designed and constructed to withstand anticipated loads. As part of the application review process, Department of Environmental Quality, Water Quality Division (DEQ/WQD) or the delegated small wastewater program shall review the design of prefabricated septic tanks for compliance with applicable

629	<u>construction standards</u> .				
630					
631	$\frac{\text{(formerly 8(a)(v))}}{\text{(ii)}}$ (ii) Installation. The septic tank shall be placed on a level				
632	grade and a firm bedding to prevent settling. Where rock or other undesirable protruding				
633	obstructions are encountered, the opening for the septic tank shall be over excavated, as needed,				
634	and backfilled with sand, crushed stone, or gravel to the proper grade.				
635					
636	(A) Septic tanks shall not be buried deeper than the tank manufacturer's				
637	maximum designed depth for the tank. The minimum depth of soil cover over the top of the tank				
638	is six (6) inches.				
639					
640	(B) <u>Backfill around and over the septic tank shall be placed in such a manner</u>				
641	as to prevent undue strain or damage to the tank or connected pipes.				
642					
643	(C) Septic tanks shall not be placed in areas subject to vehicular traffic unless				
644	engineered for the anticipated load.				
645					
646	(formerly 8(a)(ii))(iii) Size.				
647					
648	(formerly 8(a)(ii)(A)(A) Residential units serving no more than 4 families. The				
649	minimum liquid volume of <u>a</u> septic tanks shall be 1000 gallons for residences through four				
650	bedroom capacity up to a four (4) bedroom capacity. Additional capacity of 250150 gallons per				
651	bedroom shall be provided for each bedroom over four (4).				
652	<u> </u>				
653	(formerly 8(a)(ii)(B)(B) Commercial/industrial units. Septic tanks for high strength				
654	wastewater or non-residential units shall have a minimum effective liquid capacity sufficient to				
655	provide at least 36 48 hour retention at peak flow or 1,000 gallons, whichever is greater.				
656					
657	(formerly 8(a)(iii))(iv) Configuration				
658					
659	(formerly 8(a)(iii) (A)(A) The Single compartment septic tanks-shall have a length				
660	to width ratio of no less than two (2) to one (1), or be so partitioned as to provide protection				
661	protect-against short circuiting of flow. The water depth shall be no less than four feet nor greater				
662	than six feet. The inlet pipe shall be at least three inches higher than the outlet pipe.				
663					
664	(formerly 8(a)(iii) (B)(B)—If the septic tank is partitioned,—For septic tanks with two				
665	(2) compartments or more the volume of the first compartment must be at least 50 percent of the				
666	total required volume, the inlet compartment shall not be less than one-half (1/2) of the total				
667	capacity of the tank. The partition shall allow venting of the tank.				
668					
669	(C) The liquid depth shall not be less than three (3) feet nor				
670	greater than six (6) feet.				
671					
672	(formerly 8(a)(iii)(C)) The outlet elevation shall be designed to				
673	provide a distance of 20 percent of the liquid depth between the top of the liquid and the bottom				
674	of the septic tank cover for scum storage.				
675					
676	(D)—The tank partition shall allow the venting of gases				
677	between compartments and out through the vent stack on the plumbing system of the house.				

Gases generated during liquefaction of the solids are normally vented through the building's
plumbing stack vent.
(formerly 8(a)(iii)(A))(E) The-septic tank-inlet-and outlet on all tanks or tank compartments shall be provided with-a_open-ended sanitary tees-or baffles. The outlet shall be provided with a tee or baffle that extends into the middle third of the water depth to prevent floating or settled solids from carrying over into the disposal field or bed The inlet shall be provided with tee or baffle made of approved materials constructed to distribute flow and retain scum in the tank or compartments.
(I) The tees or baffles shall extend above the liquid level a minimum distance of six (6) inches.
(II) The tees or baffles shall extend below the liquid level a distance equal to thirty to forty percent (30-40%) of the liquid depth.
(III) A minimum of three (3) inches of clear space shall be provided over the top of the baffles or tees.
(formerly 8(a)(iii)(A)) (IV) The inlet pipe shall be at least_three_two (2) inches higher than the outlet pipe. (formerly 8(a)(iii)(C)) The outlet elevation shall be designed to provide a minimum_distance of _nine (9) inches or twenty (20) percent of the liquid depth, whichever is greater, between the top of the liquid and the bottom of the septic tank cover for scum storage and the venting of gases.
(v) If additional septic tank capacity over 1,000 gallons is needed, it may be obtained by joining tanks in series provided the following requirements are met:
(A) The inlet of each successive tank shall be at least two (2) inches lower than the outlet of the preceding tank, and shall have no tee or baffle except for the inlet to the first tank and the outlet for the last tank.
(B) The first tank or the first compartment of the first tank shall be equal to fifty percent (50%) or larger of the total septic tank system volume.
(formerly 8(a)(iv))(vi) Access. A manway access-riser shall be provided to each compartment of the septic tank for inspection and cleaning. A cleanout having a minimum diameter of six inches shall be provided in each tank compartment and shall extend to the ground surface and be capped.
(formerly 8(a)(iv)) (A) The manway access shall have a minimum-opening diameter-of-twenty-(20) inches in the least dimension. Both inlet and outlet devices shall be accessible.
(B) The riser shall terminate at a maximum of six (6) inches below the ground surface. Riser covers terminating above grade shall have an approved locking device.

(vii) Land application of domestic septage in remote areas that meet the conditions found in Appendix B will be permitted as a permit by rule. Delegated small wastewater programs may issue individual permits.

(viii) An effluent filter with an opening of 1/8-inch or smaller shall be provided on the outlet of a septic tank or other tank that precedes a small diameter pressure distribution system.

(formerly 8(b)) Aerobic units.

(formerly 8(b)(i)) Residential units serving no more than four dwelling units. Aerobic treatment units can be used as a pretreatment device for a single residential unit serving no more than four families provided the unit carries the seal of testing and approval from the National Sanitation Foundation (NSF) for the NSF Standard No. 40—1978. The unit shall be sized based on the flow quantities stated in Section 3. No reduction in the sizing of soil absorption systems or the final treatment systems shall be permitted if an aerobic unit is used instead of a septic tank.

(formerly 8(b)(ii))Commercial and residential units serving more than four families. Aerobic units treating wastewater generated from other than a single residential unit serving four families or less shall meet the design requirements of Part B or Part C of Chapter XI

(formerly 9(a))(b) Pumping systems for flow up to 2000 gallons per day. Dosing Tanks

(i) Pump tank. Where only one pump is provided, the pump tank shall have the minimum volume as required in Table 4 below. The <u>Dosing</u> tanks shall comply with the <u>meet the same</u> material <u>and installation</u> requirements for <u>as septic tanks</u>. The pump tank shall be vented. The vent shall have a downward turn that terminates at least 12 inches above ground and be provided with a screen. The pump tank shall have an access manhole provided with an opening at least 20 inches in least dimension. <u>Dosing tanks</u> shall have a 20-inch diameter access riser and it shall be brought to the ground <u>surface</u>.

#### Table 4

Pump Tank
Volume (gallons) Required Between

AVERAGE FLOWS (gallons per day)	<del>"OFF"</del> <del>&amp; "ON"</del> <del>SWITCH</del>	"ON" & "ALARM" SWITCH	"ALARM" SWITCH & TANK INLET	RECOMMENDED PUMP CAPACITY (gpm)
<del>0.499</del>	<del>100</del>	<del>50</del>	<del>200</del>	<del>10</del>
<del>500-999</del>	<del>200</del>	<del>100</del>	400	<del>20</del>
1000-1499	<del>300</del>	<del>100</del>	<del>600</del>	<del>30</del>
<del>1500-2000</del>	400	<del>100</del>	<del>800</del>	40

**Table 6. Dosing Tank Volume (gallons)** 

Average Design Flows (gpd)	<u>0-499</u>	<u>500-999</u>	1000-1499	<u>1500-2000</u>
Between Pump "off" and Tank Inlet	<u>350</u>	<u>700</u>	1000	<u>1300</u>
Between Tank Inlet and Alarm Switch	<u>200</u>	<u>400</u>	<u>600</u>	<u>800</u>
Between Alarm switch and Pump "on"	<u>50</u>	<u>100</u>	<u>100</u>	<u>100</u>
Between Pump "on" and Pump "off"	100	<u>200</u>	<u>300</u>	<u>400</u>
Recommended Pump Capacity (gpm)	10	20	<u>30</u>	<u>40</u>

(formerly 9(a)(ii) Pumps.

(formerly 9(a)(ii)(A) — Sizing. The pump shall have a flow rate of at least ten gallons per minute when installed. The pressure loss (feet of head) of the system can be calculated by adding: the elevation differencebetween the discharge outlet at the soil absorption system and the low water level in the pump tank; and the friction losses incurred in the pressure transfer pipe and distribution piping. Table 5 may be used to estimate the head loss of the pipe when pumping ten gallons per minute and using plastic pipe.

Table 5

<del>Diameter</del> <del>(inches)</del>	Head Loss per 100 feet of pipe (in feet)
1	<del>12</del>
11/4	4
11/2	2

(B) Installation/removal. The pump shall be installed in the tank so that it can be removed without entering the tank. This can be accomplished by (1) looping the pipe up near the access manhole with a pipe union provided at the top of the loop, (2) using a quick disconnect sliding coupler, or (3) using a pitless adapter. Chains, cable, or piping can be used to lift the pump out of the tank if designed for this loading. Setting the pump on an 8 inch block minimizes the transfer of any solids that may enter the pump tank.

(iii) Pressure transfer pipe. The pressure transfer piping between the tank and the leach system shall be designed to drain after each pump cycle to prevent freezing. This can be accomplished by either eliminating the check valve at the pump or by providing a weep hole in the pipe in the tank. If the pipe is long, the tank shall be enlarged by the volume of the pipe to accommodate the volume of liquid drained from the pipe.

(b) Syphons. Where automatic syphons are used, they shall be designed to empty the syphon tank in less than 20 minutes. The syphon tank shall be sized in accordance with Section 9(a)(i) above.

(c) For all systems exceeding 2000 gallons per day. The pumping system shall

comply with the standards of Part B of Chapter XI.

(formerly 9(a)(ii)(C)) (ii) Electrical controls. The electrical control system for the wastewater pump shall consist of a "pump off" switch, a "pump on" switch, and a "high water alarm" switch which shall be located to provide the necessary volumes as stated in Table 4. High water alarms shall be provided for all tanks that utilize pumps or siphons. The alarm device shall be an audible alarm or an indoor illuminated alarm or both. All electrical controls (pump electrical cord, switches, etc.) shall comply with the National Electrical Code—1981, Class 1, Group D, Division 1 locations. All openings around the cables or cords entering the tank shall be sealed.

- (iii) The minimum effluent level shall achieve complete submergence of the pump.
- (iv) Dosed systems using a siphon shall have a dose counter installed to check for continued function of the siphon.

### (formerly Section 12) (c) Holding tanks

(i) Holding tanks shall meet the same material requirements as septic tanks. Holding tanks shall have a twenty (20)-inch minimum diameter access riser. The riser shall be brought to ground surface.

(formerly 12(a)) (ii) Uses. Holding tanks shall not be used for residential systems when other alternative systems are available, except on a temporary, seasonal or intermittent basis, or when used to correct a failed subsurface disposal soil absorption system when other alternatives are unavailable. Use of holding tanks for new construction is prohibited.

(formerly 12(b)) Acceptance. A letter of verification from the local receiving agency, denoting acceptance of the wastewater generated shall be submitted with the plans.

(iii) (formerly 12(c)) Location. The location and construction of holding tanks shall meet the requirements for septic tanks in Sections 4(a)(i) and Section 8(a)(i) respectively. Holding tanks must be located in an area readily accessible to the pump truck and where the tank itself will not float due to a high groundwater. If seasonal high groundwater may be present, the tank shall be properly anchored.

(formerly 12(a)) (iv) Where holding tanks are allowed, they shall be sized on the basis of seven days storage at the flow rate determined from Table 1. The minimum liquid volume shall be the greater of 1,000 gallons or seven (7) days storage based upon flow rate determined from Section 4.

(formerly 12(d)) Vent. Each holding tank shall be provided with a two inch minimum diameter vent ending in a return elbow above final grade. The vent shall terminate at least 30 feet from any door, window, or fresh air inlet. The vent should be screened.

(formerly 12(e)) (v)—Alarm. All holding tanks shall be equipped with a high water level alarm. The device shall be an audible alarm or an indoor illuminated alarm or both. The

25-26

alarm level shall be placed device shall be installed so that the alarm is triggered when the water level reaches at 3/4 of the depth of the tank capacity.

(formerly 12(f)) Pumpout. A six inch pump out pipe which extends to the surface shall be provided. It shall be capped at all times.

(vi) A design package for holding tanks is provided online at the Division's website to assist the applicant in submitting a completed application for coverage under the general permit for small wastewater systems. The worksheet and calculations were prepared by a registered professional engineer employed by the Wyoming Department of Environmental Quality, Water Quality Division. The general design requirements stated in this section are incorporated into the worksheets such that by properly completing the forms and installing the components, the system will comply with these requirements.

(formerly 9(c)(d) Grease Interceptors – grease, oil, silt and sand.

- (i) When required. Liquid wastes containing grease, oil, or silt and sand A commercial or institutional food preparation facility with a waste stream containing fat, oil, and grease (FOG) in excess of 25 mg/L shall provide install an exterior grease interceptor or a device approved by the delegated health department or county before the septic tank. Waste streams from residential living units are exempt from this requirement. Facilities that typically have waste streams high in FOG are, but not limited to, restaurants, cafeterias, slaughterhouses, and institutional kitchens.
- (ii) Material. The interceptor shall meet the material requirements of Section 8(a)(i). Waste streams high in FOG shall be plumbed separately and directly to a grease interceptor prior to the waste treatment process.
- (iii) Waste streams from sanitary facilities such as bathrooms, toilets, urinals, or other similar fixtures shall not be discharged into the grease interceptor. These sources must be connected at least four to six (4-6) feet downstream of the grease interceptor's discharge. The design shall prevent any backflow from the sanitary sources into the grease interceptor.
  - (iv) Only one source facility per grease interceptor shall be allowed.

(formerly 8(c)(v)) Access. The access shall meet the requirements of Section 8(a)(iv).

(formerly 8(e)(vi)) Location. (v) Grease interceptors shall be located so that they are easily accessible for inspection, cleaning, and removal of the collected wastes.

Interceptors shall be placed as close as practical to the fixture it serves. The wastewater from fixtures not producing grease, oil, or sand and silt shall bypass the interceptor. The interceptor shall not be closer than fifteen (15) feet from the last discharging fixture and no further away than thirty-five (35) feet.

(formerly 8(c)(iv)) (vi) Configuration. Grease-interceptors shall have a\_minimum at least of two (2) compartments with the first compartment having at least 50 percent of the total required volume a 20-inch minimum diameter clean out riser for each compartment. Each compartment shall be vented. Each clean out riser shall be brought to the surface and have a

	(vii) The	re sh	all be	no inte	rnal	cleanout tees	or by	oasse	<u>S.</u>			
	( · · · · · · · · · · · · · · · · · · ·				0.4				. 1 701			
						_	_		oe vented. The			
<u>sha</u>	ll be at least two (2	) 1nc	hes in	diamet	er.	he inlet and	<u>outlet</u>	vents	shall not be 11	nterco	nnected.	
	<i>(</i> 1.) <b>77</b>							40				
		outl	et pipe	<u>invert</u>	shal	l be no more	than t	wo (2	) inches lower	than_	the inlet	
nv	<u>ert.</u>											
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								II be	the same heigh	it as t	ne other	
val	lls and the cover mu	ıst c	<u>ontact</u>	the top	of th	<u>ne dividing w</u>	all.					
					_	_						
						_			n from the bot		f a riser	
_	e that terminates at	leas	t eight	<u>een (18</u>	<u>3) inc</u>	hes below the	<u>e inlet</u>	pipe	invert of that s	same		
con	<u>npartment.</u>											
				_		be accessible	during	g nori	<u>nal business h</u>	ours v	<u>vithout</u>	
nte	errupting normal bu	sine	ss oper	<u>rations.</u>	<u>.</u>							
									with the man		<u>ırer's</u>	
	tructions and applic											
	tructions shall be su	<u>ıbmi</u>	tted w	ith ever	ry pe	rmit to const	uct ap	<u>plica</u>	tion submitted	to		
DE	Q/WQD.											
	(formerly	<del>8(c)</del>	<del>(iii))</del> (z	xiv) (	Treas	e interceptor	s shall	be si	zed <del>using one</del>	of the	<u>,</u>	
<u>acc</u>	(formerly ording to the follow (A)	ving <u>Tł</u>	formu	<del>las</del> : <u>imum v</u>	<u>volun</u>	ne shall not b	e less	than	zed using one	of the	<del>)</del>	
acc	ording to the follow	ving <u>Tł</u>	formu	<del>las</del> : <u>imum v</u>	<u>volun</u>	-	e less	than	_	of the	•	
acc	ording to the follow (A)	ving <u>Tł</u>	formu	<del>las</del> : <u>imum v</u>	volun	ne shall not b	e less llowir	than	750 gallons.	of the	<del>)</del>	
<u>acc</u>	ording to the follow (A)	ving <u>Tł</u>	formu	<del>las</del> : <u>imum v</u>	volun	ne shall not b	e less llowir	than	750 gallons.	of the	•	
acc	ording to the follow (A) (B)	ving <u>Tł</u>	formune min	las: imum v sized a	volun	ne shall not be ding to the fo	e less llowir	than ng: (grea	750 gallons. se. garbage)			
	ording to the follow (A) (B)  Number of meals	ving <u>Tł</u>	formune minuall be	imum v sized a	ccore Con	ne shall not be ding to the formercial kite	e less llowir	than  ng:  (grea	750 gallons.  se. garbage)		Interceptor siz	<del>ze(liquid</del>
	ording to the follow (A) (B)	ving Th	formune minuall be	las: imum v sized a	volun	ne shall not be ding to the fo	e less llowir	than  ng:  (grea	750 gallons. se. garbage)			<del>ze(liquid</del>
Γ	ording to the follow (A) (B)  Number of meals	ving Th	formune minuall be	imum v sized a	ccore Con	ne shall not be ding to the formercial kite	e less llowir	than  ng:  (grea	750 gallons.  se. garbage)		Interceptor siz	ze(liquid
Γ	ording to the follow (A) (B)  Number of meals	ving Th	formune minuall be	imum v sized a	ccore Con	ne shall not be ding to the formercial kits  Retention time**  Car  wash (sand, silt, oil)	e less llowir ehens	than  ng:  (grea	750 gallons.  Se. garbage)  age  or***	=	Interceptor size capacity)	
	ording to the follow  (A)  (B)  Number of meals per peak hour	Th Sh	formune minuall be	imum v sized a	ccore Con	ne shall not be ding to the formercial kits  Retention time**  Car  wash (sand, silt,	e less llowir ehens	than  ng:  (grea	750 gallons.  se. garbage)	=	Interceptor siz	or size

sealed lid that is rated for any anticipated load. There shall be a means provided to sample the

944 945 946 947					Laund s (gre- lint, s	ase.						
	Number of 2 cyc		· ·	<sup>z</sup> aste ow rate	<b>X</b>	Retention time	X	Storage factor	Ð	=	Interceptor capacity	size (liquid
948 949 950 951	*Waste flow rate - see Table 1.											
951 952 953	** Retention Ti	mes										
755		Co	mmercial kite	<del>chen w</del>	aste:							
			Dishwashe	<del>r and/o</del>	<del>r dispos</del>	<del>al</del>		2.5	houi	<del>'S</del>		
		Sir	igle service k	<del>itchen:</del>								
			Single serv	ing wit	h dispos	<del>sal</del>		1.5	houi	<del>:S</del>		
		Ca	<del>r washers</del>					2.0	houi	<del>'S</del>		
		La	<del>undries</del>					2.0	houi	<del>:S</del>		
954 955 956	***Storage Facto	<del>rs</del>										
	Fully equipped comm	ercial	kitchen					<del>16</del>	hr. o	peration: 1 peration: 2 peration: 3		
_	Single service kitcher	<del>1</del>									1.5	
	Carwashers										serve: 1.5	
_	*										perated: 2	
957	Laundries							1.5 (al	H <del>OW!</del>	<del>s for r</del>	<del>rock filter)</del>	
957 958 959			Commerc	<del>cial</del> Kit	tchens (	grease, gart	oage)					
	Number of meals per peak hour	<u>X</u>	Waste Flow rate*	<u>X</u>	Retent time**		Storag factor				eptor size d capacity)	
960 961	*Waste flow rate -	see T	able 2.									
962 963 964	**Retention times											
		]	Kitchen waste	<u>e:</u>								
		_	Dishwasher		_	<u>al</u> 2.5	hours					
		<u> </u>	Single service Single serv			sal 15	hours					
965 966	***Storage factors	L <u>-</u>	Single serv	ing wit	<u>ir dispo</u>	11.5	Hours					
967	E.,11.	equi-	oed commerc	ial			Q he	operatio	n: 1	7		
	<u>kitche</u>		ged commerc	<u>141</u>				operatio				

							2	24 hr. opera	tion	• 3
		Single ser	vice k	titchen:				z i iii. opere		1.5
968		211310 001	71001							
969	<u>(e)</u>	Other Inter	rcento	rs						
970	357	<u> </u>	- CO D CO	<u> 10</u>						
971		(i) Inter	cento	rs are required	l for oi	il. grease, sai	nd and	other subs	tance	es harmful or
972	hazardous t			ainage system,						
973									,	
974		(A)	Lau	<u>ndries</u>						
975		<del></del>								
976			(I)	Commercial	l laund	lries, Laundr	omats.	, and dry-c	leane	ers shall be
977	equipped w	ith an interd	ceptor	in order to red	duce tl	ne quantity o	of lint a	and silt that	ente	er the
978	collection s	system.	•			•				
979										
980			<u>(II)</u>	The system	must ł	oe of adequa	te size	and design	to a	allow for cool-
981	down of wa	astewater so	that s	separation can	be mo	re readily ac	chievec	<u>1.</u>		
982										
983			(III)							
984				ng, that prever						
985			size, s	string, rags, bu	ittons,	or other mat	erials	<u>detrimenta</u>	to t	<u>he waste</u>
986	treatment s	<u>ystem.</u>								
987			(TV)	G:-:	1			a fallanda	- <b>6</b>	1
988 989			<u>(1V)</u>	Sizing must	be in	accordance \	with th	<u>le following</u>	<u>g TOT</u>	<u>muia:</u>
999										
				Laundr	ios (ar	ooso lint si	ile)			
991				<b>Laundr</b>	ies (gr	ease, lint, si	<u>ilt)</u>			
						1		Storage		
991	Total gallons	per cycle	<u>X</u>	Cycles per	<u>ies (gr</u>	Retention	<u>X</u>	Storage factor**	=	<u>Interceptor</u>
991 992	Total gallons	per cycle	<u>X</u>			1		Storage factor**	=	Interceptor
991			<u>X</u>	Cycles per		Retention			Ξ	Interceptor
991 992 993	Total gallons *Retention		<u>X</u>	Cycles per		Retention			Ξ	Interceptor
991 992 993 994				Cycles per	<u>X</u>	Retention	<u>X</u>	factor**	Ξ	Interceptor
991 992 993 994			Insti	Cycles per hour	<u>X</u>	Retention time*		factor**	=	Interceptor
991 992 993 994			<u>Insti</u> <u>Stan</u>	Cycles per hour	<u>X</u> ries  cial lau	Retention time*	<u>X</u>	factor**  ours  ours	=	Interceptor
991 992 993 994			<u>Insti</u> <u>Stan</u>	Cycles per hour  tutional laund dard commerce	<u>X</u> ries  cial lau	Retention time*	2.5 ho 2.0 ho	factor**  ours  ours	=	Interceptor
991 992 993 994 995		times	<u>Insti</u> <u>Stan</u>	Cycles per hour  tutional laund dard commerce	<u>X</u> ries  cial lau	Retention time*	2.5 ho 2.0 ho	factor**  ours  ours	=	Interceptor
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991 992 993 994 995 996 997	*Retention	times  factors  8 hours of	Insti Stan Ligh	Cycles per hour  tutional laund dard commercial et commercial	ries vial lau laundr	Retention time*	2.5 ho 2.0 ho	factor**  ours  ours		1.0
991 992 993 994 995 996 997 998	*Retention	times  factors  8 hours of	Insti Stan Ligh	Cycles per hour  tutional laund dard commercial	ries vial lau laundr	Retention time*	2.5 ho 2.0 ho	factor**  ours  ours		
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991 992 993 994 995 996 997 998 999 1000 1001	*Retention	factors  8 hours of 12 or mor	Insti Stan Ligh	Cycles per hour  tutional laund dard commercial atton  ation rs of operation  Washes	ries cial lau laundr	Retention time*	2.5 ho 2.0 ho 1.5 ho	factor**  ours  ours  ours	<u>]</u>	1.0 1.5
991 992 993 994 995 996 997 998 999 1000 1001 1002	*Retention  **Storage	factors  8 hours of 12 or mor  (B)	Insti Stan Ligh Copera e hour	Cycles per hour  tutional laund dard commercial extreme to operation  Washes  Where autor	ries cial lau laundr	Retention time*	2.5 ho 2.0 ho 1.5 ho	factor**  ours  ours  ours  ours  ours  ours	<u>]</u>	1.0 1.5
991 992 993 994 995 996 997 998 999 1000 1001 1002 1003	*Retention  **Storage f	factors  8 hours of 12 or mor  (B)	Insti Stan Ligh Car (I) separa	Cycles per hour  tutional laund dard commercial ation ation washes Where autorators shall have	ries cial lau laundi	Retention time*	2.5 ho 2.0 ho 1.5 ho	factor**  ours  ours  ours  ours  ours  ours	<u>]</u>	1.0 1.5
991 992 993 994 995 996 997 998 999 1000 1001 1002 1003 1004	*Retention  **Storage f	factors  8 hours of 12 or mor  (B)	Insti Stan Ligh Car (I) separa	Cycles per hour  tutional laund dard commercial extreme to operation  Washes  Where autor	ries cial lau laundi	Retention time*	2.5 ho 2.0 ho 1.5 ho	factor**  ours  ours  ours  ours  ours  ours	<u>]</u>	1.0 1.5
991 992 993 994 995 996 997 998 1000 1001 1002 1003 1004 1005	*Retention  **Storage f	factors  8 hours of 12 or mor  (B)	Insti Stan Ligh Car (I) separa gallor	Cycles per hour  tutional laund dard commercial extreme to commercial extreme with the commercial extreme extreme with the commercial extreme	ries vial laundr laundr mobile e a mit	Retention time*  undry  y  es are washenimum capa ery other bay	2.5 ho 2.0 ho 1.5 ho d (inclucity of	ours ours ours ours ours ours ours ours	1 1 sho	ops utilizing or the first bay,
991 992 993 994 995 996 997 998 999 1000 1001 1002 1003 1004	**Storage f	factors  8 hours of 12 or mor  (B)  practices), ditional 500	Insti Stan Ligh Car (I) separa gallor (II)	Cycles per hour  tutional laund dard commercial ation ation washes Where autorators shall have	ries cial lau laundi	Retention time*  undry  y  es are washed nimum capa ery other bay	2.5 ho 2.0 ho 1.5 ho d (inclucity of	ours ours ours ours ours ours ours ours	l sho	ops utilizing or the first bay,

1008 structures with at least two walls and appropriate grading to prevent stormwater infiltration into 1009 the sanitary sewer. 1010 1011 (III) An effluent sampling point is required. 1012 1013 (f) Abandonment of Septic and Holding Tanks 1014 The following is the procedure to abandon septic tanks and holding tanks when the system is 1015 upgraded, equipment replacement is necessary, or central sewer lines are made available. 1016 1017 The abandoned tank should be pumped and the septage hauled to a licensed 1018 facility approved to receive the waste or the septage pumped into the newly constructed septic or 1019 holding tank. Discharging to a central sewer requires coordination with, and the approval of, the 1020 owner/operator of the sewer system. 1021 1022 (ii) Once the abandoned tank is empty, it should be removed and the excavation 1023 backfilled. As an alternative to removing the tank, the access covers can be removed and the tank 1024 filled with native soil, pit run, or sand. 1025 1026 (iii) If the abandoned tank is part of a Class V UIC facility, the abandonment must 1027 also be in compliance with Chapter 16, Section 12. 1028 1029 Section 10. Subsurface Treatment and Disposal Systems Effluent Distribution Devices. 1030 1031 Distribution boxes and flow divider tees are suitable for level or nearly level ground and are 1032 installed before the soil absorption system with the goal of splitting flows equally between soil 1033 absorption system laterals. Drop boxes are suitable for sloping ground and are installed to 1034 achieve serial loading. 1035 1036 **Distribution Boxes** (a) 1037 1038 (formerly 10(a)((vii)(i) Distribution box. If a The distribution box is used, it shall 1039 be installed to provide uniform distribution of the wastewater on a level, stable base to ensure 1040 against tilting or settling and shall be placed so that it will not be subject to and to minimize 1041 movement from frost heave. 1042 1043 (ii) Boxes shall be watertight and constructed of concrete or other durable material. 1044 1045 (iii) Boxes shall be designed to accommodate the inlet pipe and the necessary 1046 distribution lines. The inlet piping to the distribution box shall be at least one (1) inch above the 1047 outlet pipes and all pipes shall have a watertight connection to the distribution box. 1048 1049 The box shall be protected against freezing and made accessible for (iv) 1050 observation and maintenance. 1051 1052 Boxes shall have flow equalizers installed on each outflow. (v) 1053 1054 (b) Flow divider tees may be used in place of distribution boxes. 1055

(c) Drop boxes are suitable for sloping ground and are installed to achieve serial loading. The drop boxes shall meet the requirements in paragraphs (a)(i through v) of this section. Section 11. Evapotranspiration Beds Standard Soil Absorption Systems. (a) Sizing. The area of evapotranspiration beds shall be determined using the following formula: where: Area = Area of the evapotranspiration bed at the ground surface in square feet = Average daily sewage flow, gallons per day, (0.6 times the flow determined from Table 1) PET = Potential evapotranspiration rate in inches per year P = Annual precipitation rate in inches per year. (b) Construction. If an impervious barrier is necessary for the protection of groundwater it shall be installed between the evapotranspiration bed and the native soil. It shall be a polyvinyl chloride sheet with a minimum thickness of 20 mils or equivalent. A 3 inch layer of sand shall be placed under and over the liner. (ii) The bottom 12 inches of the bed shall be filled with clean stone 1/2 - 2 1/2 inches in (iii) Perforated pipe complying with Section 10(a)(v) shall be placed in the stone. (iv) Four inches of pea gravel (less than 1/4 inch in diameter) or durable filter cloth shall be placed over the stone. (v) A 24 inch uniform sand layer in the size range of D50 (0.10mm) shall be placed on top of the pea gravel or filter cloth. (vi) A six inch layer of sandy topsoil shall be placed on top of the evapotranspiration bed. (vii) The bed should be vegetated with small shrubs and/or grasses such as fescue, brome, or alfalfa. (viii) The evapotranspiration bed shall be placed at a depth sufficient to prevent surcharging of the septic tank. (formerly 10(a) (a) General Design #Requirements:

1102 (i) All soil absorption systems shall be designed in such a manner that the effluent 1103 is effectively filtered and retained below ground surface. The absorption surface accepts, treats, 1104 and disperses wastewater as it percolates through the soil. 1105 (formerly 10(a)(ii)(ii) Protection. Effort shall be made to protect the natural absorptive properties of the soil. Soil absorption systems shall not be installed during adverse 1106 1107 weather or soil conditions. Rain, severely cold temperatures, or excessively moist soils are 1108 considered adverse weather or soil conditions. All smeared or compacted surfaces shall be 1109 restored to their original infiltrative conditions prior to placement of the stone. Soil absorption 1110 systems shall not be excavated when the soil is wet enough to smear or compact easily. Open soil 1111 absorption system excavations shall be protected from surface runoff to prevent the entrance of 1112 silt and debris. All smeared or compacted surfaces shall be raked to a depth of one (1) inch, and 1113 loose material removed before filter or filler material is placed in the soil absorption system 1114 excavation. 1115 1116 (formerly 10(a)(ii) Runoff. Surface runoff shall be diverted around or away from all 1117 soil absorption systems. 1118 1119 (iii) Soil absorption systems shall be designed to approximately follow the ground 1120 surface contours so that variation in excavation depths will be minimized. The trenches may be 1121 installed at different elevations, but the bottom of each individual trench shall be level throughout 1122 its length. 1123 1124 (formerly 10(a)(ix)) (iv) Earth cover. A minimum of 12 inches of earth shall be placed over the absorption system stone. The earth shall be permeable soil that will allow aeration 1125 of the system and will support the growth of grass. The earth cover shall be graded to insure that 1126 1127 water will not pond on the surface. Shallow soil absorption system depths are encouraged to 1128 promote treatment and evapotranspiration. The minimum soil cover depth over the soil 1129 absorption system is one (1) foot. The maximum depth to the bottom absorption surface of a soil 1130 absorption system is five (5) feet. Finished grading shall prevent ponding and promote surface 1131 water runoff. 1132 1133 (v) Pipes, chambers or other products shall be bedded on firm, stable material. 1134 Heavy equipment shall not be driven in or over soil absorption systems during construction or 1135 backfilling. 1136 1137 (vi) Standard trenches refer to perforated pipe embedded in aggregate-filled 1138 trenches which shall conform to the following: 1139 1140 (formerly 10(a)v))(A) Gravity pipe. All plastic gravity absorption system The perforated pipes-shall have a minimum diameter of four 4 inches and shall conform to ASTM 1141 standard D2729. Suitable pipe materials include: ASTM D-2729-11 PVC, ASTM D-3034-08 1142 1143 PVC, Schedule 40 PVC ASTM d1784-11, and ASTM F810-07 PE. Piping in all horizontally

constructed absorption systems shall be layed with the holes centered around the vertical axis at the bottom of the pipe. All field tile pipe shall be spaced 1/4 inch apart. Piping in horizontally constructed absorption systems shall have a maximum slope of three inches per 100 feet.

(formerly 10(a)(vi)) Pressure pipe. All pressure distribution piping shall be designed to withstand the anticipated pressures with a safety factor of two, provide uniform application of the wastewater, and have non-clogging orifices.

(formerly 10(a)(iv)) (B)—Stone. Soil absorption system stone The aggregate shall be crushed rock, gravel or other acceptable, durable and inert material which is free of fines, sized and has an effective diameter between 1/2-inch to 2 1/2inches. At least two inches of stone shall be placed over the distribution pipe, and at least six inches of stone shall be placed under and beside the distribution piping. A minimum of 12 inches of stone shall be placed between a seepage pit wall and structural liner. The stone shall be free from sand, silt, and clay.

(formerly 10(a)(viii))(C) Stone cover. A suitable cover such as untreated building paper, filter cloth, or straw shall be placed over the stone prior to backfilling the system. Prior to backfilling, the aggregate shall be covered throughout with a woven/non-woven geotextile material or a three (3) inch layer of straw.

(D) Aggregate shall extend the full width and length of the soil absorption system to a depth of at least twelve (12) inches with at least six (6) inches of drain gravel under the distribution pipe and at least two (2) inches over the distribution pipe.

### (E) Maximum width of trench excavation is three (3) feet.

(formerly 10(d))(F)—Special requirements for trench systems. A Minimum separation spacing of trenches(wall to wall) of is three (3) feet or a horizontal distance equal to 1.25 times the vertical depth of the trenches, whichever is greater, of undisturbed soil shall be maintained between adjacent trench sidewalls. Trench spacing shall be increased to nine (9) feet when the area between each trench is considered as reserve area. For clay loam soils that have percolation rates greater than 60 min/in., the nine (9) foot spacing shall also be required but it is not considered as reserve area.

(formerly 10(f))(vii) Special requirement for bed systems. The distribution system piping shall be spaced no more than 10 feet apart. Standard beds shall conform to the same pipe and aggregate requirements for trenches as found in subparagraphs (vi)(A through D) of this section. Standard beds shall also conform to the following:

(formerly 10(a)(x)) (A) The soils shall have percolation rates less than 60 minutes per inch (5-60 mpi). Levelness. The bottom of soil absorption systems and each segment of a sidehill system the bed shall must be level, therefore the site shall be relatively flat, sloping no more than one (1) foot from the highest to the lowest point in the installation area.

_	(B) Distribution laterals within a bed must be spaced on not greater than six
(6) feet centers.	Sidewalls shall not be more than three (3) feet from a distribution lateral.
	(C) Beds must not be wider than twenty-five (25) feet if gravity distribution
is used. Multiple	e beds must be spaced at one-half the bed width.
	(D) Rubber tired vehicles must not be driven on the bottom surface of any
ed excavation.	
(viii)	Chambered trenches, when used in lieu of perforated pipe and aggregate, shall
	nformance with the manufacturer recommendations. No cracked, weakened,
	rwise damaged chamber units shall be used in any installation.
	(A) All chambers shall be an open, arch-shaped structure of durable, non-
legradable design	n, suitable for distribution of effluent without filter material.
	(B) All chamber endplates shall be designed so that the bottom elevation of
the inlet nine is a	t least six (6) inches from the bottom of the chamber.
me met pipe is u	treast six (b) menes from the bottom of the chamber.
	(C) Inlet and outlet effluent sewer pipes shall enter and exit the chamber
endplates. Inspec	ction ports shall be installed at all outlet effluent sewer pipes.
	(D) All chambers shall have a splash plate under the inlet pipe or another
lesign feature to	avoid unnecessary channeling into the trench bottom.
	(E) Maximum width of trench excavation is three (3) feet.
	(F) Minimum spacing of trenches (wall to wall) is three (3) feet. Trench
spacing shall be i	ncreased to nine (9) feet when the area between each trench is considered as
	clay loam soils that have percolation rates more than 60 min/in., the nine (9)
	l also be required but it is not considered as reserve area.
<u>(ix)</u>	Chambered beds shall conform to the same requirements for chambered
trenches as found	l in subparagraphs (viii)(A through D) of this section. Aggregate, as specified in
subparagraph (vi)	)(B) of this section, or native soil shall be used to fill the space between the
chambers.	
(5	
	nerly 10(e)(x) Special requirements for serial sidehill trench or bed systems.
Serial Sidehill Tr	ench:
	(formerly 10(e)(i)) (A)—Separation. A minimum of three six (6) feet of
undisturbed soil s	shall be maintained between adjacent trench or bed side walls.
	·

1230 (formerly 10(e)(ii))(B) Levelness. The bottom of each serial trench or bed 1231 system shall be level. 1232 1233 (formerly 10(e)(iii))(C) Overflow. The overflow pipe between serial soil 1234 absorption systems shall be set no higher than the mid-point of the upstream distribution pipe. 1235 The overflow pipe shall not be perforated. 1236 1237 (formerly 10(b) Special requirements for seepage pits. If a structural lining is needed to 1238 support stone in a seepage pit, it shall be constructed of durable material not subject to excessive 1239 corrosion or decay and structurally capable of supporting the loads to which it will be subjected. 1240 The lining shall be perforated or otherwise designed to allow the passage of wastewater. Seepage 1241 pits shall be separated by a minimum distance equal to 3 times their diameter. 1242 1243 (b) A design package for standard soil absorption systems is provided online at the 1244 Division's website to assist the applicant in submitting a completed application for coverage 1245 under the general permit for small wastewater systems. The worksheet and calculations were 1246 prepared by a registered professional engineer employed by the Wyoming Department of 1247 Environmental Quality, Water Quality Division. The general design requirements stated in this 1248 section are incorporated into the worksheets such that by properly completing the forms and 1249 installing the components, the system will comply with these requirements. 1250 1251 Section 12. Holding Tanks Pressure Distribution Systems. 1252 1253 (a) **General Design Requirements:** 1254 1255 The basic elements of a pressure distribution system include a dosing tank, 1256 filter, and a means to deliver specified doses to a small diameter pipe network within a soil 1257 absorption system. Pressure distribution is required for mound systems or for bed systems with a 1258 width greater than twenty-five (25) feet. 1259 1260 (ii) Pumps must be sized to match the distribution system curve or demand. 1261 Pumps shall be designed for sewage pumping applications and be accessible from the ground 1262 surface. 1263 1264 (iii) The control system for the pump and dosing tank shall, at a minimum, consist of a "pump off" switch, a "pump on" switch, a "high liquid alarm". 1265 1266 1267 (A) All electrical connections must be made outside of the chamber in either 1268 an approved weatherproof box or an explosion-proof junction box. 1269 1270 (B) The wiring from the junction box to the control box must pass through a 1271 sealing fitting to prevent corrosive gases from entering the control panel. 1272 1273 (C) All wires must be contained in solid conduit from the dosing chamber to 1274 the control box.

1276 (iv) The pressure transport piping between the tank and the soil absorption system 1277 shall be designed to prevent freezing. 1278 1279 (A) The ends of lateral piping shall be constructed with long sweep elbows or an equivalent method to bring the end of the pipe to finished grade. The ends of the pipe shall be 1280 1281 provided with threaded plugs, caps, or other devices to allow for access and flushing of the 1282 lateral. 1283 1284 (B) All joints in the manifold, lateral piping, and fittings shall be solvent-1285 welded using the appropriate joint compound for the pipe material. Pressure transport piping 1286 may be solvent-welded or flexible gasket jointed. 1287 1288 (C) Where automatic siphons or other devices are used, they shall be 1289 designed to empty the dosing tank in less than ten (10) minutes. 1290 1291 (v) The pressure distribution system shall have a combination of at least three (3) 1292 vertical feet of filter sand and/or unsaturated native soil above the high groundwater level. The 1293 filter sand shall conform to ASTM C-33, with less than 2% passing the #200 sieve. 1294 1295 A design package for pressure distribution systems is provided online at the 1296 Division's website to assist the applicant in submitting a completed application for coverage 1297 under the general permit for small wastewater systems. The worksheet and calculations were 1298 prepared by a registered professional engineer employed by the Wyoming Department of 1299 Environmental Quality, Water Quality Division. The general design requirements stated in this 1300 section are incorporated into the worksheets such that by properly completing the forms and 1301 installing the components, the system will comply with these requirements. 1302 1303 Section 13. Privies Sand Mound Systems. 1304 1305 The sand mound consists of a sand fill, an aggregate bed and a soil cap. 1306 1307 (a) Selection Criteria: 1308 1309 The high groundwater level, bedrock or impervious clay layer is less than four (4) feet below the 1310 bottom of the soil absorption system excavation. 1311 1312 (b) Site Requirements: 1313 1314 (i) A minimum of one (1) foot of vertical separation of the native soil is required 1315 between the bottom of the sand fill and the top of the high groundwater level, any restrictive 1316 layer, or any highly permeable material. 1317 1318 (ii) The percolation rate of the native soil at the interface of the sand fill shall be 1319 greater than five (5) and less than sixty (60) minutes per inch (5-60 mpi). The percolation shall 1320 be measured in the top twelve (12) inches of native soil. 1321

(formerl	y 10 <del>(c))</del> (c)—	Special re	equirements for	mounded system	ms. General Design
Requirements:	•	•		•	
_					
(fo	ormerly 10(c) <del>(i)</del>	) Sizing(i)	Sand Layer		
				stone and the fill n	
			•	<del>l the allowable loa</del>	•
•	_		*	of the fill. The to	
surface is the s	<del>um of the sidew</del>	all and botto	<del>m areas of the s</del>	<del>tone - soil interfac</del>	ee below the
<del>distribution pip</del>	<del>oe.</del>				
	(B) The inte	<del>rface area be</del>	<del>tween the fill so</del>	<del>il and the native s</del>	oil shall be sized
<del>based on the in</del>	filtration rate of	the native so	oil as determine	d by Figure 7 of S	ection 38 but shall
not be smaller	<del>than a system d</del>	esigned to the	<del>e requirements c</del>	of subsection (ii) b	<del>elow.</del>
	•	-	-		
	(A) Filter	sand shall co	nform to ASTM	I C-33, with less t	han 2% passing the
#200 sieve.					
	(B) The m	ninimum dept	th of sand below	the aggregate be	d surface shall be
one (1) foot.	<u>(= / = == = = = = = = = = = = = = = = = </u>				
<u> </u>					
	(formerly 10	(c)(ii))( <b>C</b> )	Grade The f	inished grade shal	l extend at least
three feet hori				to the parent soil	
				nd shall have a co	
					e high groundwater
level.					
	(I)	For sand m	nounds using pre	essure distribution	systems, the depth
o high ground	water shall be th	ree (3) feet b	elow the botton	n of the absorption	n surface if the
percolation rate	e of the soil is fi	ve (5) minute	es per inch or gr	eater (5-60 mpi).	
		<u> </u>			
	(D) The to	p of the sand	l layer under the	aggregate bed sh	all be level in all
directions.	, , ===================================	1	.,		
	(E) The sa	and layer chal	ll fill around the	perimeter of and	to the top of the
agragata bad		mu iayti silal	ii iiii arouna tile	permieter of allu	to the top of the
aggregate bed.					
	(E) (E) 1	C 11 11		(2) 1 - 4 1 :	(1) (1 1
er	(F) The sl		1 11 1 -1	e (3) horizontal to	one (1) vertical or
<u>flatter.</u>		ope of all sid	les shall be three	(3) HOHEOHUU to	one (1) vertical of
		ope of all sid	les shall be three	o (5) norizontar to	one (1) vertical of
	•	)(c) <del>(i)(B))</del> (G)	<u>)</u> The <del>-interfac</del> e	e- <u>infiltration</u> area-	between the fill soil
	soil which is the	0(c) <del>(i)(B))</del> (G)	) The <del>-interfac</del> ne sand fill shall	e- <u>infiltration</u> area- be sized-calculate	between the fill soil

1365 flowrates (gpd) from Table 1 or Table 2 by the loading rate (gpd/ft<sup>2</sup>) found in Table 5. but shall 1366 not be smaller than a system designed to the requirements of subsection (ii) below. 1367 1368 (ii) Aggregate Bed 1369 1370 (A) The aggregate shall be crushed rock, gravel or other acceptable, durable 1371 and inert material which is free from fines, and has an effective diameter between one-half (1/2) 1372 inch and two and one half  $(2 \frac{1}{2})$  inch. 1373 1374 (B) The aggregate bed depth shall not be less than nine (9) inches with a 1375 minimum of six (6) inches of clean aggregate placed below the distribution pipe and two (2) 1376 inches above the distribution pipe. The aggregate shall be covered with an approved geotextile 1377 material after installation and testing of the pressure distribution system. 1378 1379 The design shall be a long, narrow bed design with a maximum width of 1380 twenty-five (25) feet. 1381 1382 (D) The infiltration area, which is the bottom of the aggregate bed, shall be 1383 calculated by dividing the design flowrates (gpd) from Table 1 and Table 2 by the loading rate of 1384  $0.8 \text{ gpd/ft}^2$ . 1385 1386 (iii) Soil Cover 1387 1388 (A) The soil cap shall be constructed of a sandy loam, loamy sand, or silt 1389 loam. The depth of the soil cap shall be at least six (6) inches at the edges to twelve (12) inches 1390 at the center. The slope of all sides shall be three (3) horizontal to one (1) vertical or flatter. 1391 1392 (formerly 10(c)<del>(iii))</del>(B) Fill soil. The fill soil that is A layer of top soil at 1393 least six (6) inches thick shall be placed between the native soil and the stone over the entire sand 1394 mound area. shall have a minimum percolation rate of five minutes per inch. Topsoil shall be 1395 placed over the mound to promote vegetative cover. The sand mound should be planted with 1396 vegetation that does not require watering and will not establish deep roots. Native grasses are 1397 commonly used. 1398 1399 (formerly 10(c)(iv)) Preparation. All trees, roots, and other organic matter shall be 1400 removed from the area to be occupied by the mound. 1401 1402 (d) A design package for sand mound systems is provided online at the Division's 1403 website to assist the applicant in submitting a completed application for coverage under the 1404 general permit for small wastewater systems. The worksheet and calculations were prepared by a 1405 registered professional engineer employed by the Wyoming Department of Environmental 1406 Quality, Water Quality Division. The general design requirements stated in this section are

1407 incorporated into the worksheets such that by properly completing the forms and installing the 1408 components, the system will comply with these requirements. 1409 1410 Section 14. Chemical Toilets Small Wastewater Lagoons. 1411 (formerly 14(a) General requirements. Chemical toilets shall only be used in the 1412 1413 containment of body wastes. These requirements apply only to the use of chemical toilets for 1414 permanent structures. 1415 1416 (formerly 14(b) Greywater. If indoor plumbing is installed, a separate greywater 1417 disposal is required and shall meet the requirements of Section 3 through 12. The minimum 1418 design flows for greywater shall be obtained from Table 1 with a reduction of 33 percent 1419 allowed for the elimination of blackwater wastes. 1420 1421 (formerly 14(c) Disposal. All chemical toilet wastes shall be disposed of at an 1422 approved wastewater facility. A letter of verification from the receiving agency, denoting 1423 acceptance of the wastewater generated shall be submitted with the plans. These wastes shall 1424 not be discharged into a soil absorption system. 1425 1426 (formerly 14(d) Construction. Chemical toilets shall be constructed and installed to 1427 resist breakage or damage from routine usage. Outdoor chemical toilets shall be adequately 1428 stabilized and secured to prevent overturning. Materials used shall be resistant to the sewage 1429 wastes and the chemicals encountered. The holding compartment of the toilet shall be constructed to prevent accessibility to the public and to disease transmitting vectors. 1430 1431 1432 (formerly 14(e) Additives. No chemical or biological additive shall be placed in the toilet that may adversely affect the operation of a sewage treatment facility where the toilet 1433 1434 waste will ultimately be disposed or that may adversely impact the quality of the groundwater 1435 as specified in Chapter VIII, "Quality Standards for Groundwater of Wyoming." 1436 1437 (formerly 15(a))(a) General requirements. Selection Criteria: 1438 1439 (formerly 15(a)(i)) (i) The use of this section for small nondischarging waste 1440 stabilization ponds applies only to those systems defined as small wastewater systems. All other 1441 treatment systems shall meet the requirements of Part B or Part C of Chapter XI as applicable. 1442 Lagoons shall only be considered in areas of Wyoming where the annual evaporation 1443 exceeds the annual precipitation during the active use of the lagoon. 1444 1445 (formerly 15(a)(ii)) (ii) Non-discharging waste stabilization ponds-Lagoons shall 1446 only be constructed in soils allowed where when the percolation rate exceeds sixty (60) minutes 1447 per inch and the soil is at least 1 foot thick on both the sides and bottom of the pond-extends 1448 vertically down at least two (2) feet from the bottom of the lagoon to the seasonal high 1449 groundwater table or bedrock formations. If the 60 minute per inch percolation rate cannot be 1450 obtained, a sufficient clay shall be incorporated into the top foot of soil until the 60 minute per 1451 inch percolation rate is reached. An impermeable artificial liner of 20 mils in thickness may be 1452 substituted. 1453 1454 (iii) A lagoon shall not be constructed within the 100 year flood plain.

1456 (b) General Design Requirements: 1457 1458 (formerly 15(b)) (i) Isolation. The isolation distances shall meet the requirements for 1459 absorption systems as specified in Section 4(a)(i). Beyond the horizontal setback distances 1460 requirements specified in Section 6(d) of this rule, the lagoon shall not be placed within one 1461 hundred (100) feet of the owner's property line. 1462 1463 (ii) The use of a septic tank which meets the specifications in Section 9 of this rule 1464 shall be required before the small wastewater lagoon. 1465 1466 (iii) The lagoon shall be located and constructed so it will not receive surface runoff 1467 water. 1468 (iv) The slope of the lagoon site shall not exceed five percent (5%). 1469 The lagoon site must be located in an area of maximum exposure to sun and 1470 wind. 1471 1472 (vi) The lagoon shall be designed for complete retention. 1473 1474 (formerly 15(d)) Sizing. (vii) The area of the lagoon shall be calculated based on 1475 the following formula. 1476  $A = \frac{584 \times Q}{(365 \times S) + (E - P)} \times 1.3$ 1477 1478 1479 A = Area of the lagoon (in square feet) at the maximum operating depth of 5 feet feet 1480 water level in square feet 1481 1482 O = Average daily sewage flow, gallons per day. (0.6 times the flow determined from 1483 Table 1) (Multiply values from Table 1 or 2 by 0.6 to get average daily flow.) 1484 1485 E = Average annual lake evaporation rate in inches per year. (Note: lake evaporation is 1486 less than pan evaporation; lake evaporation equals pan evaporation times a pan coefficient of 0.7) 1487 1488 P = Average annual precipitation rate in inches per year. 1489 S = Soil permeability in inches per day "S" cannot be greater than 0.25 inches per day 1490 1491 "S" shall equal zero for an artificial liner or for bedrock Seepage rate in decimal form, in inches 1492 per day. 1493 (formerly 15(e)) Construction requirements. 1494 1495 1496 (formerly 15(e)(i)(viii) The slopes of the inside dikes shall not be steeper than 1497 three-(3) horizontal to one(1) vertical-nor flatter than four horizontal to one vertical. The slopes 1498 of the outside dikes shall not be steeper than three horizontal to one vertical and shall not allow 1499 surface runoff to enter the pond. (formerly 15(e)(iv)) The minimum top width of the top of the 1500 dike shall be eight four (4) feet.

1501 1502 (formerly 15(e)(iii)) (ix) All fill-material shall consist of impervious material that is 1503 well compacted and free of rocks, frozen soil, or other large material. 1504 1505 (x) (formerly 15(d)(ii)) A The minimum water level operating depth of at least two 1506 feet shall be two (2) feet maintained in the pond at all times, including start up. (formerly 1507 15(d)(iii)) A minimum free board of two feet shall be provided between the lowest embankment 1508 berm and the maximum water level. The maximum water level shall not be less than five feet. The 1509 dikes shall provide a minimum freeboard of two (2) feet. 1510 1511 (formerly 15(e)(ii)) (xi) All organic material and debris shall be removed from the 1512 pond site prior to construction. The floor of the lagoon shall be level and maintained free of all 1513 vegetation. 1514 1515 (xii) The influent line into the lagoon must discharge near the center. 1516 1517 (xiii) A cleanout or manhole shall be provided in the influent line near the dike. 1518 1519 (xiv) The area around the small wastewater lagoon shall be fenced to preclude the 1520 entrance of livestock, pets, and humans. The fence shall be equipped with a locking gate. The 1521 gate shall have a sign indicating "NO TRESPASSING - WASTEWATER LAGOON". 1522 1523 A design package for a small wastewater lagoons is provided online at the Division's 1524 website to assist the applicant in submitting a completed application for coverage under the 1525 general permit for small wastewater systems. The worksheet and calculations were prepared by a 1526 registered professional engineer employed by the Wyoming Department of Environmental 1527 Quality, Water Quality Division. The general design requirements stated in this section are 1528 incorporated into the worksheets such that by properly completing the forms and installing the 1529 components, the system will comply with these requirements. 1530 1531 (formerly 15(c)) Groundwater protection and bedrock or impermeable soil separation. 1532 1533 (formerly 15(c)(i)) For single family homes, the depth to seasonally high 1534 groundwater shall be at least four feet from the bottom of pond. 1535 1536 (formerly 15(c) (ii)) For all "small wastewater systems" other than single family homes, a minimum of three feet of unsaturated soil shall be maintained between the bottom of the 1537 1538 pond and the estimated groundwater mound imposed on the seasonally high groundwater table. The height of the groundwater mound can be estimated from Figures 1-6, Section 5 in 1539 1540 conjunction with the average daily sewage flow. 1541 1542 Section 15. Small Non-Discharging Waste Stabilization Ponds Privies. 1543 1544 Pre-fabricated privies and outhouses shall be sealed, water-tight vaults shall meet the following 1545 conditions. 1546 1547 (formerly 13(a)) General requirements. 1548

1550 shall meet the requirements of Section 3 through 12. The minimum design flow for grey water 1551 shall be obtained from Table 1 with a reduction of 33 percent allowed for the elimination of black 1552 wastes. 1553 1554 (formerly 13(a) (iii) The privy shall consist of a vault and an outhouse building. 1555 1556 (formerly 13(b))(a) Isolation. The isolation horizontal setback distance requirements 1557 for sealed privies shall comply with Section 6(g) for septic tanks. 1558 1559 (formerly 13(d)(ii))(b) The depth to seasonally high groundwater from the bottom of a 1560 water tight vault shall be sufficient to prevent floatation of the empty vault. 1561 1562 (formerly 13(c)) Soil exploration. Soil exploration to a minimum depth of 4 feet below 1563 the bottom of the proposed vault shall be made to provide information on subsoil condition. 1564 1565 The vault must have sufficient capacity for the dwelling served, and must have at 1566 least 27 cubic feet or 200 gallons of capacity. 1567 1568 (formerly 13(a)(i))(d) All privies shall be designed and constructed to prevent access 1569 by flies and rodents. The privy must be easily maintained and insect tight. The door must be self-1570 closing. The privy seat must include a cover. All exterior openings, including vent openings, 1571 shall be screened. 1572 1573 (formerly 13(d)) Groundwater and bedrock separation. 1574 1575 (formerly 13(d)(i)) The depth to seasonally high groundwater and bedrock or impermeable soil shall be at least four feet from the bottom of an unlined vault. 1576 1577 1578 (formerly 13(e)) Sizing. Vaults shall have a minimum capacity of 500 gallons per riser 1579 and shall be a minimum of 4.5 feet deep. 1580 1581 (formerly 13(f)) Construction. 1582 1583 (formerly 13(f)(i)) The vault shall be constructed and installed to resist breakage and 1584 damage imposed by frost heave, uplift pressures from a fluctuating water table, loads imposed by 1585 the outhouse building and soils, and damage that may be caused by vandalism or rough cleaning 1586 procedures. The vault shall be constructed 1587 to prevent access by flies. 1588 1589 (formerly 13(f)(ii)) Materials used for vault construction shall be resistant to alkali 1590 attack, hydrogen sulfide gas, and other corrosive elements associated with decomposing waste. 1591 1592 (formerly 13(f)(iii)) A clean-out manhole shall be installed and shall have a 1593 minimum opening of 20 inches in the least dimension. The manhole shall be located outside of 1594 the outhouse building and be equipped with a tightfitting secure cover. 1595 1596 (formerly 13(f)(iv))(e) Privies must be adequately vented.

(formerly 13(a) (ii) If indoor plumbing is installed, the grey water disposal method

The vault shall be ventilated to a point outside and above the outhouse building. The outhouse building shall have a set of vents installed near the floor on two opposite sides of the building and a roof vent that has a rain cap. All vents shall be screened.

(formerly 13(g)) Vault additives. No chemical or biological additive shall be placed in the vault that may adversely effect the operation of a sewage treatment facility where the vault waste will ultimately be disposed or that may adversely impact the quality of the groundwater as specified in Chapter VIII, "Quality Standards for Groundwater of Wyoming".

(f) Privies shall not be constructed within the 100 year flood plain.

 (g) A design package for privies is provided online at the Division's website to assist the applicant in submitting a completed application for coverage under the general permit for small wastewater systems. The worksheet and calculations were prepared by a registered professional engineer employed by the Wyoming Department of Environmental Quality, Water Quality Division. The general design requirements stated in this section are incorporated into the worksheets such that by properly completing the forms and installing the components, the system will comply with these requirements.

## Section 16. Commercial/Industrial Wastes. Greywater Systems.

It is the intent of this section to encourage and facilitate the productive and safe reuse of greywater from domestic wastewater.

(a) Applicability

(i) This section applies to any person who utilizes greywater for beneficial irrigation uses.

(ii) This section is not applicable if the intent is to provide blackwater treatment.

(iii) A city, county, or other local government agency may, after a public hearing and enactment of an ordinance or resolution, further restrict or prohibit the use of greywater systems.

(b) Greywater Operation and Requirements

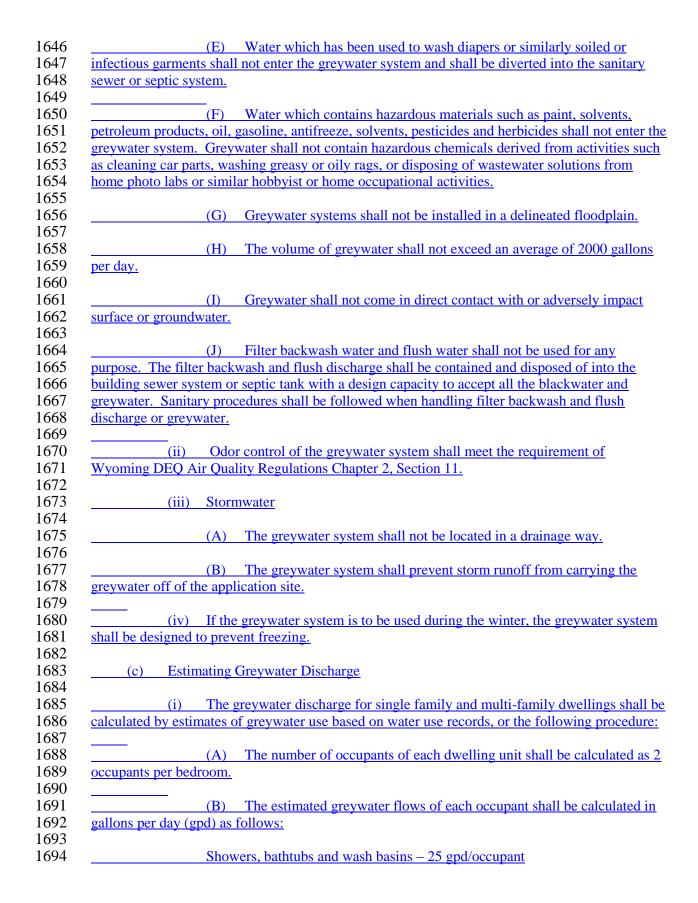
(i) Restrictions

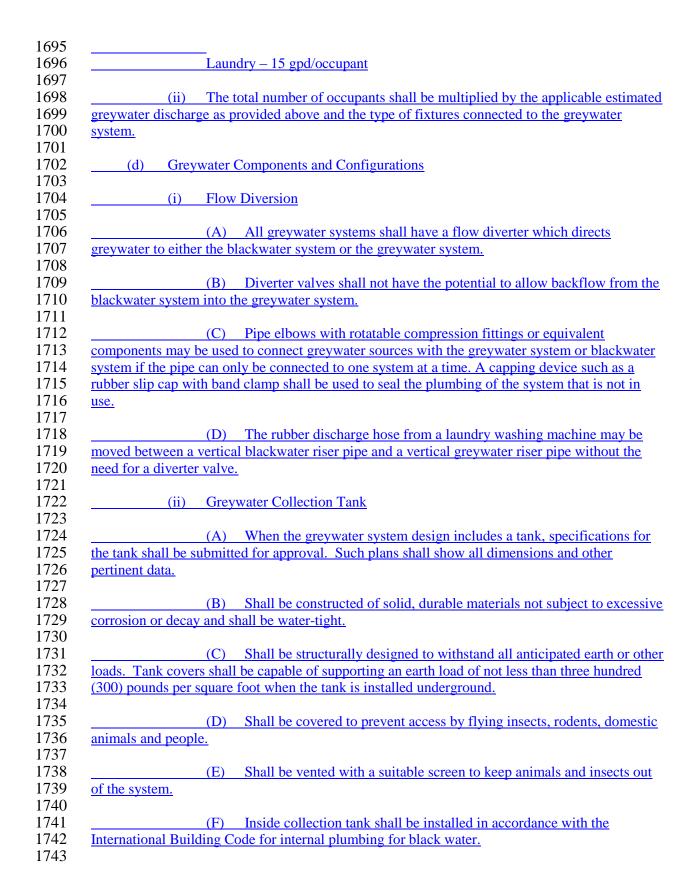
(A) Spray irrigation of greywater is not permitted.

(B) The installation of a greywater system shall not reduce or alter the sizing requirements of the onsite wastewater system.

(C) Human, domestic pets, and animal contact with greywater and soil irrigated with greywater shall be minimized.

(D) Greywater shall not leave the property on which it is generated. Ponding or runoff is prohibited.

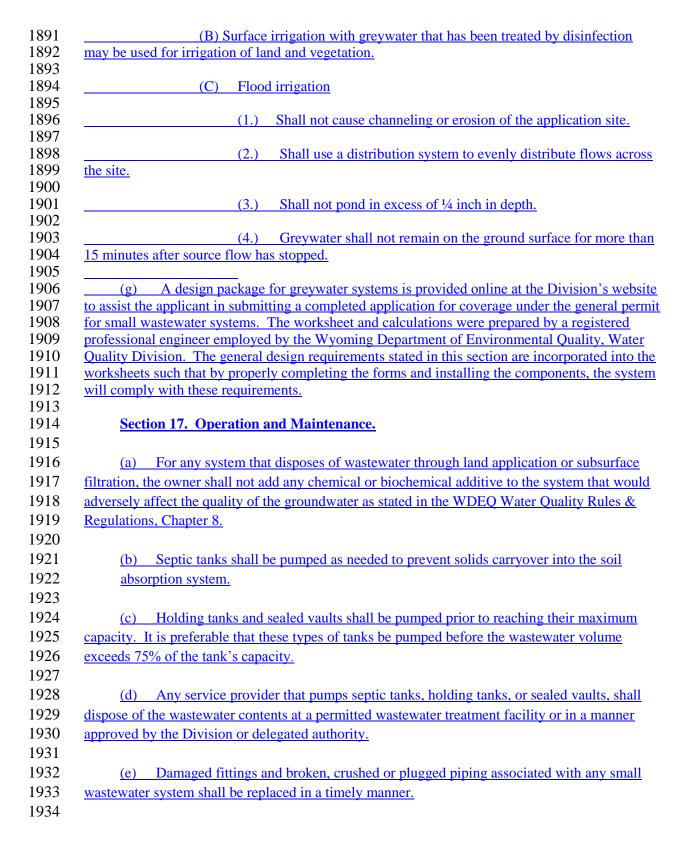




	(G) Shall not hold greywater for more than 24 hours.
	(H) Overflow Requirements:
	(I) Each tank shall have an overflow drain. The overflow drain
	a permanent connection to the building drain or building sewer, upstream of septic
tanks, if an	y. The overflow drain shall not be equipped with a shutoff valve.
	(II) The overflow drain shall not be less in diameter than the inlet
pipe.	
	(III) The overflow system must be designed so that the tank overflow
	by gravity to the existing sewer line or septic tank. The tank shall be protected against
sewer line	backflow by a check valve.
	(A) Craywatar agnyayanga pinas shall be pappanantly labeled for
Greywater	(A) Greywater conveyance pipes shall be permanently labeled for or shall be colored purple. Non-paint marking pens are unacceptable as permanent
<u>labeling.</u>	or shan oc colored purple. Ivon-paint marking pens are unacceptable as permanent
	(B) Gravity flow pipes shall be constructed to allow complete draining of
the pipe.	
	(C) Pressurized pipe systems shall be constructed and designed to be
drained or	the water evacuated by compressed air for winterization.
	(iv) Disinfection
	(iv) Districction
	(A) All greywater to be used for surface irrigation shall be disinfected.
	(B) Disinfection may be accomplished through chemical methods or
ultraviolet	disinfection systems.
	(I) Chemical disinfection
	(1.) Chemical disinfection methods include the use of iodine,
chlorine, or	bromine.
	(2.) Chemical disinfection shall provide the proper dosage of
disinfection	n to achieve a fecal coliform level of 200/100 mL or less.
	(II) Ultraviolet disinfection systems
	(1.) Ultraviolet (UV) disinfection systems shall be designed
and installe	(1.) Ultraviolet (UV) disinfection systems shall be designed according to the manufacturer recommendations.
are mount	to the management recommendations.
	(2.) Greywater disinfected by a UV disinfection system shall
have a HV	transmittance less than the UV transmittance rated by the manufacturer.

be exceeded	(3.) The max flow rate of the UV disinfection system shall 1.
<u>(e)</u>	Setbacks
	(i) A 30 foot buffer zone is required between the greywater application site
	operty lines and any public right-of-way. This buffer zone requirement may be m
the use of a	subsurface drip irrigation system.
	(ii) A 30 foot separation distance is required between greywater application
and all surf	
	<del></del>
and all note	(iii) A 100 foot separation distance is required between greywater application ble water supply wells.
and an pota	ble water suppry werrs.
(f)	Greywater Applications.
	_ (i) Conord
	(i) General
	(A) Each zone of an irrigation field must be of adequate size to receive
greywater a	nticipated in that zone.
	(B) No irrigation or disposal field shall extend within three (3) vertica
of the highe	est known seasonal groundwater, or to a depth where greywater contaminates the
groundwate	er or surface water.
	(C) Permeable pipe systems designed for greywater shall be installed
according to	o manufacturer's recommendations.
	<del>-</del>
	(ii) Subsurface Irrigation
	(A) Subsurface irrigation with greywater may be used to irrigate land
vegetation.	
days after a	(B) Food crops for direct human consumption shall not be harvested to pplication of greywater.
days arter a	
	(C) Subsurface irrigation shall not overwhelm the absorption system
leading to o	overland flow.
	(D) Mulch Basins
agual to the	(1.) The total irrigation and/or mulch basin area required must b
equal to the	estimated greywater discharge (gpd) divided by the absorption capacity (gpd/ft2
	(2.) Shall be sized to provide sufficient depth, length and width

	periodic maintenance, reshaping or removal of dirt to maintain surge capacity, e plant growth, and prevent ponding or runoff.
	(3.) Shall not be deeper than the root zone of the plants to be
irrigated.	(3.) Shan not be deeper than the root zone of the plants to be
	(4.) Free Flow Outlets
	(4.) Thee flow Outlets
	a. Greywater shall be applied at the top of the mulch.
	b. Application point(s) shall be protected from access by
	s, rodents, domestic animals and people. Protections shall be constructed to allow or cleaning and maintenance.
easy access to	or cleaning and maintenance.
1.1.1	c. Inlet piping to the mulch basin shall be no less than 1 inc
nigher than ti	he surface to which it is applied to allow for free fall of water.
	(5.) Sub-mulch Outlets
	a. Greywater shall be applied below the surface of the mule
into one or m	ore distribution chambers constructed of perforated material.
	b. Inlet piping to distribution chamber of the mulch basin
shall be no le	ess than 2 inches higher than the surface to which it is applied to allow for free fall
water.	Distribution should be 11 be a sector of for some
cleaning and	c. Distribution chamber shall be constructed for easy maintenance.
	(6.) A compost pile shall meet the requirements of a mulch basin.
	(E) Drip Systems
	(1.) Shall be filtered prior to the point of application or shall be
designed to p	revent frequent clogging.
application o	(2.) Discharge nozzles shall be specifically designed for the f greywater without clogging.
<u>apprount</u>	
in diameter.	(3.) Drilled pipe drip system holes shall be no smaller than ¼ inche
in utailletel.	
C CI	(4.) Point of application flow shall be low enough to prevent any
surface flow	or greywater.
	(iii) Surface Irrigation
	(A) Greywater used for surface irrigation shall receive a level of
disinfection s	so the maximum fecal coliform level is 200/100 mL or less.



(f) Composting or non-discharging toilets where permitted shall have their waste disposed of at a permitted wastewater treatment facility or landfill, or in a manner approved by the Division or delegated authority.

Section 18. Commercial and Industrial Wastes and/or Domestic Wastes Greater than 2000 Gallons Per Day.

(formerly 16 (a)) (a) General requirements. Those Commercial/industrial wastewater systems or combination commercial/industrial and domestic wastewater systems are subject to applicable requirements listed in Section 1 through 12 14 and 15 of this chapter, in addition to requirements in this section.

(formerly 16(b)) (b) Hydrogeologic investigation.—If the wastewater is classified as, or determined to be hazardous and/or toxic and/or contain petroleum products, the applicant shall demonstrate to the administrator that any discharge or seepage from the wastewater facility will not cause a violation of the surface and/or groundwaters of the state in accordance with Chapter 1, "Quality Standards for Wyoming Surface Waters" and Chapter 8, "Quality Standards for Wyoming Groundwaters." Due to the wide variety of wastes, wastewater and site conditions, the latest available scientific information shall be used to demonstrate that violation will not occur.

(formerly 16(c)) (c) — Impact. If the impact of the hazardous and/or toxic substance and/or petroleum products cannot be determined and mitigated, disposal of the wastewater using a soil absorption system shall be prohibited.

(formerly 16(d)) (d) Pre-treatment. Pre-treatment of the wastewater to remove the hazardous, and/or toxic, substance(s) and/or petroleum products shall be required prior to disposal if deemed necessary to protect the groundwater and surface water(s) of the state.

(formerly 4(a)(ii)) (e) If the flow is greater than 2000 gpd but less than 10,000 gpd, the minimum isolation distances (in feet) shown in Table 3 shall be maintained. The minimum horizontal setback distances (in feet) shown in Table 7 shall be maintained for commercial and industrial wastes and/or wastes greater than 2000 gallons per day but less than 10,000 gallons per day.

Table 7. Minimum Horizontal Setbacks for Commercial and Industrial Wastes<sup>1</sup>

From	To Septic Tank Or Equivalent	To Absorption System
Wells (includes neighboring wells)	50	200
Public Water Supply Well	<u>100<sup>2</sup></u>	<u>500²</u>
Property Lines	10	10
Foundation Wall (w/o drains)	5	10
Foundation Wall (with drains)	5	50
Potable Water Pipes	25	50
Septic Tank	N/A	10
Stream or Surface Body of Water, Spring (including seasonal and intermittent)	50	100
<u>Cisterns</u>	<u>50</u>	<u>50</u>

<sup>&</sup>lt;sup>1</sup> (formerly 4(a)(iii)) For systems larger than 10,000 gallons per day, the isolation distance shall be determined by a hydrogeological study in accordance with Section 15(b) Section 17(b) of Chapter III, but shall not be less than those in subsection two above shown in Table 7.

<sup>2</sup> Wastewater systems that discharge to the same aquifer that supplies a public water supply well and are located within Zone 1 or 2 (Attenuation) of the public water supply well, as determined by Wyoming Department of Environmental Quality Source Water Assessment Project (2004) or as established in Section 2 of the Wyoming Wellhead Protection Guidance Document (1997), shall provide additional treatment. These systems will be required to obtain an individual permit to construct and will require that a PE sign, stamp, and date the application, as stated in Section 2 of this chapter. The additional treatment shall be in accordance with Chapter III, Section 2(b)(ii). The treatment shall reduce the nitrates to less than 10 mg/L of NO<sub>3</sub>, as N and provide 4-log removal of pathogens before the discharge leaves the property boundary of each small wastewater system.

## 1987 APPENDIX A 1988 **Percolation Test Procedure** 1989 1990 Section 1. Purpose 1991 1992 (a) Percolation tests are used to determine absorption system site suitability and to size 1993 the absorption system. 1994 1995 Section 2. Procedure 1996 1997 **Location.** General Requirements: (formerly (a)) (a) 1998 1999 (i) Percolation tests shall not be conducted in test holes which extend into 2000 groundwater, bedrock, or frozen ground. 2001 2002 (ii) The percolation test shall be conducted only after the soil exploration pit has 2003 been dug and examined. 2004 2005 (formerly (a)) (iii) A minimum of three (3) percolation test holes are required. 2006 2007 (formerly (a)) (iv) The percolation test holes shall be spaced uniformly over the 2008 proposed soil absorption system site. 2009 2010 (formerly (b)) (b) Preparation. 2011 2012 (formerly (b))(i) A four (4) inch to twelve (12) inch diameter hole shall be dug or 2013 bored to the proposed depth of the soil absorption field system. 2014 2015 (ii) The walls shall be vertical, with the natural soil surface exposed without 2016 smearing. 2017 2018 (iii) To expose a natural soil surface The sides and bottom shall be scraped scarified 2019 with a sharp pointed instrument and the loose material shall be removed from the hole. 2020 2021 (iv) Two (2) inches of Coarse sand or gravel gravel or coarse sand shall be placed 2022 in the bottom of the hole to prevent it from scouring and sealing during water addition. 2023 2024 (c) Presoaking 2025 2026 (formerly (c)) (i) Presoaking. The purpose of presoaking is to have the water 2027 conditions in the soil reach a stable condition similar to that which exists during continual 2028 wastewater application. The minimum time of presoaking varies with soil conditions but must 2029 be sufficiently long so that the water seeps away at a constant rate. The following presoaking 2030 instructions are usually sufficient to obtain a constant rate. 2031 2032 (formerly (c)(i)) (A)—In sandy soils, place 12 inches of water in the hole Fill 2033 each hole with clear water to a level at least eighteen (18) inches above the gravel or coarse 2034 sand and allow it to seep away. Fill the hole again with 12 inches of water and if the water 2035 seeps away in ten minutes or less, it indicates that the soil is excessively permeable and

requirements in Section 5(d) of these regulations shall be followed. If the eighteen (18) inches of water seeps away in eighteen (18) minutes or less, add eighteen (18) inches of water a second time. If the water remains after ten minutes, additional saturation is necessary. Refer to Appendix A(e)(ii) below. If the second filling of eighteen (18) inches of water seeps away in eighteen (18) minutes or less, this indicates the soil is sandy and is excessively permeable. The soil absorption system shall meet the requirements of Section 7 (c).

(formerly (c) (ii)) (B)—In other soils, maintain 12 inches of water in the hole for at least four hours.—If either the first or second fillings of eighteen (18) inches of water does not seep away in ninety (90) minutes, eighteen (18) inches of water must be maintained in the hole for at least four (4) hours to presoak the test hole. After the four (4) hours of water contact time, allow the soil to swell for wait at least twelve (12) hours-before starting the percolation rate measurement as stated in Appendix A (d) below.

(formerly (d) (d) Percolation Rate Measurement

The water level should be adjusted to six inches above the gravel initially and after each time interval measurement when necessary.

(formerly (i))(i) In other soils, establish a fixed reference point and measure the drop in water level at constant intervals. The water level drop should be measured to the nearest 1/8 of an inch. The test may be terminated when the water drop is consistent for three consecutive measurements. Fill each test hole with twelve (12) inches of water and allow the soil to rehydrate for 15 minutes prior to any measurements

- (ii) Establish a fixed reference point to measure the incremental water level drop at constant time intervals. The water level drop should be measured to the nearest ½ of an inch and the minimum time interval is ten (10) minutes.
- (iii) Refill the test hole to twelve (12) inches above the gravel before starting the measurements. Continue to measure the incremental water level drop at a constant time interval until a consistent incremental water level drop is achieved. A consistent water level drop is achieved when three (3) consecutive water level drops are within ½ inches of each other.
- (iv) Before the water level drops below one (1) inch above the gravel, refill the test hole to twelve (12) inches and continue to measure the incremental water level drop.

 $\frac{\text{(formerly d(ii))}(v)}{\text{for each hole}}$  is calculated as follows for each hole using the following formula:

Time Interval (Minutes) = Percolation Rate Final Water Level Drop (inches) (minutes/inch)

(formerly d(ii))\_(vi) \_\_\_\_ If only three to five percolation tests are performed, the design percolation rate for the absorption system is the slowest rate from all the holes tested. If six or more percolation tests are performed, the design percolation rate for the absorption system is the average of all the holes tested as determined by the above formula.

(e) The following information shall be recorded:

2082	(i) Date(s) of test(s);
2083 2084	(ii) Location, diameter, and depth of each test hole;
2085 2086	(iii) Duration of presoak;
2087 2088	(iv) Time of day for beginning and end of each water-level drop interval;
2089 2090	(v) Each water-level drop measurement;
2091 2092	(vi) Calculated percolation rate;
2093 2094	(vii) Name and signature of person performing test;
2095	
2096 2097	(viii) Name of owner or project name; and
2098 2099	(ix) Certification that the percolation test was done in accordance with Wyoming Water Quality Rules and Regulations Chapter 25 Appendix A.
2100	

2101 2102 (Formerly Chapter 15, Appendix C) APPENDIX B 2103 **General Statewide Permit** 2104 For Land Application of Domestic Septage in Remote Areas 2105 2106 Department of Environmental Quality/Water Quality Division 2107 **Septage Land Application Worksheet** 2108 2109 **Section 1. Restrictions and Requirements** 2110 2111 To qualify for the land application of domestic septage (domestic septage being defined as either 2112 liquid or solid material removed from a septic tank result from normal household wastes) in 2113 remote areas, the following conditions must be met. 2114 2115 **DEFINITIONS** 2116 \* "Permanent waterbody" means perennial streams, lakes, wetlands, etc. that have water 2117 throughout the year 2118 \* "Intermittent stream" means a stream or part of a stream that is below the local water 2119 table for some part of the year but is not a perennial stream. 2120 2121 2122 \* "Ephemeral stream" means a stream which flows only in direct response to precipitation 2123 in the immediaste watershed or in response to snow melt, and has a channel bottom that 2124 is always above the prevaling water table. 2125 2126 "Wetland" means those areas having all three essential characteristics: (A) Hydrophytic vegetation; 2127 2128 2129 (B) Hydric soils; 2130 2131 (C) Wetlands hydrology. 2132 2133 (a) Location restrictions 2134 2135 (i) Only domestic septage generated on the property owner's location may be land 2136 applied on the same property owner's location. Domestic septage generated on a specific property 2137 may be land applied on said property, and shall not be transported to another location for land 2138 application. 2139 2140 (ii) A minimum distance of at least 1,000 feet must be maintained from all adjacent 2141 properties No land application of domestic septage shall occur within 1,000 feet of all adjacent 2142 properties. 2143 2144 (iii) No land application of domestic septage may occur within 300 feet of a 2145 permanent waterbody, intermittent stream, ephemeral stream or wetland. 2146 2147 No land application of domestic septage may occur within 300 feet of public road. 2148

•	—No land application of domestic septage shall occur within 300 feet of a public road,
permaner	at surface water body, or intermittent stream.
	No land application of domestic sewage may occur within 1000 feet of a residence
(b)	Site restrictions:
<u>(U)</u>	Site restrictions.
	(i) The land application of domestic septage shall only occur on those sites with
establishe	ed vegetation such as rangeland, pasture or hay meadows.
	(ii) No more than 5,000 gallons of domestic septage per acre per year shall be land
pplied.	
	(iii) No land application of domestic septage may occur where the depth from the
<del>surface to</del>	groundwater is less than four (4) feet.
	No land application of domestic septage may occur where site slopes exceed five
<del>percent (</del>	
five perce	No land application of domestic septage shall occur where the site's slope exceeds ent (5%) or where the depth to groundwater is less than four (4) feet.
irve perce	the (370) of where the depth to groundwater is less than four (4) feet.
	(iv) The land application of domestic septage shall not occur between November 1
and May	1, or any other time when frozen or saturated ground conditions exits.
	(v) No public access shall be allowed to any site where domestic septage has been
applied fo	or at least one (1) year following application. to any site where domestic septage has
<del>been app</del> l	ied.
	Lime stabilization of the septage to pH 12 for 30 minutes prior to land application is
<del>optional</del>	
•	(vi) No grazing animals shall be allowed access to any site where domestic septage
	land applied for at least thirty (30) days following application. to any site where
<del>domestic</del>	septage has been land applied.
<u>(c)</u>	Crop restrictions:
	(i) No root crops shall be harvested <u>from soils where domestic septage has been</u>
	ied for at least thirty-eight(38) months following application. from soils where domestic
<del>septage n</del>	as been land applied.
	(ii) No truck crops (harvested parts touch land surface) shall be harvested from
soils whe	re domestic septage has been land applied for at least fourteen(14) months following
application	on. from soils where domestic septage has been land applied.
	(iii) No commodity arong (other food food and fiber arong whose horsested roots
do not to	(iii) No commodity crops (other food, feed, and fiber crops whose harvested parts uch land surface) shall be harvested <u>from soils where domestic septage has been land</u>
	or at least thirty(30) days following application. from soils where domestic septage has
<del>been land</del>	

2198	
2199	(iv) No turf shall be harvested from soils where domestic septage has been land
2200	applied for at least one(1) year following application. from soils where domestic septage has been
2201	land applied.
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2203	(d) Reporting Requirements:
2204	
2205 2206 2207 2208 2209	(i) The property owner shall notify the appropriate Department of Environmental Quality, Water Quality Division (DEQ/WQD) District Office Engineer prior to the land application of domestic septage to confirm the requirements and to arrange a possible DEQ/WQD inspection of the land application.
2210	(ii) All records concerned with each septage application will be maintained for at
2211	least five (5) years.
2212	icast five (5) years.
2213	(iii) There is a worksheet provided online at the Division's website that must be
2214	completed, signed and returned to the DEQ/WQD or the appropriate delegated local permitting
2215	authority within 15 days of the land application.
2216	autionly within 10 days of the faile approunding
2217	This worksheet must be completed, signed, and returned to the Department of
2218	Environmental Quality, Water Quality Division or the appropriate delegated local permitting
2219	authority within 15 days of the land application.
2220	
2221	Provide the following information concerning your site. Enter NA if not applicable.
2222	
2223	1. Date of the application:
2224	2. Number of acres receiving septage:
2225	3. Number of gallons of septage land applied:
2226	4. Type of vegetation receiving:
2227	5. Name, address and telephone number of septage hauler:
2228	
2229	
2230	<u> </u>
2231	
2232	-6. If septage was optionally alkali stabilized, please indicate what material
2233	was used for stabilization and how pH was measured:
2234	
2235	
2236	7) Please indicate that the site sketch on the back of this sheet has been
2237	completed and complies with the site restriction distances yes/no:
2238	
2239	8) Please indicate if photos of the land application site will be sent to the
2240	appropriate District Office: Yes/no
2241	9) Please provide physical address or legal description of land application
2242	site:
2243	10) Plana dia dia mana 64 PEO/WOD
2244	10) Please give the name of the DEQ/WQD representative contacted, and
2245	time and date. This contact needs to be made prior to the domestic septage land
2246	application:

		) 1,000 feet from adje ) 1,000 feet from any ) 300 feet frot water, inte str drainage
ı		REQUIRED DISTA F APPL ) 1,000 feet from adju
I	STE STEICH	

ignature of applicator	
<del>Name (printed)</del>	